

2015 IBC LATERAL ENGINEERING

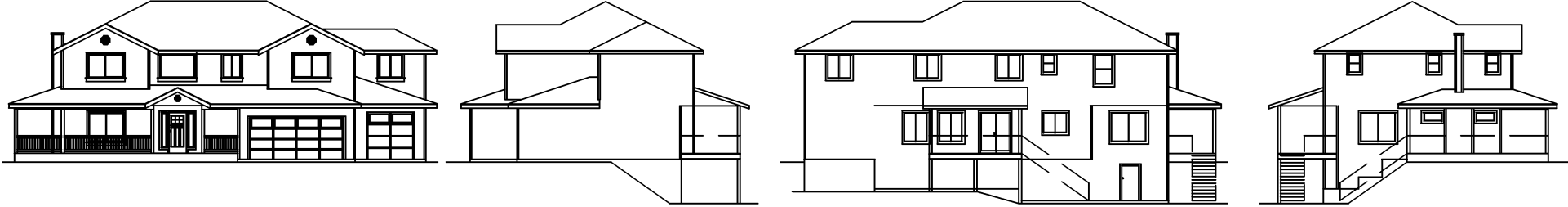
PROJECT NUMBER: 180558  
FOR: FERGUSON & COLE, INC  
PLAN: 2800

CHANGES  
MUST BE APPROVED PRIOR  
TO PERFORMING WORK  
08/02/2018 11:56:17 AM  
# 18-02763

SITE CRITERIA

DESIGN ROOF SNOW LOAD	WIND SPEED (V-ULT)	WIND EXPOSURE	(IBC) SEISMIC DESIGN CATEGORY	SITE SOIL CLASS.	RISK CATEGORY	SLOPE PER OWNER	SPECIAL SITE CONDITIONS PER OWNER	GEOTECH REPORT PER OWNER	FROST DEPTH	SEISMIC SPECTRAL RESPONSE S <sub>1</sub>	SEISMIC SPECTRAL RESPONSE S <sub>s</sub>	APPROX. ELEVATION
25 PSF	110 MPH	B	D	D	II	<15%	NONE	NONE	12"	0.60g	1.40g	280 FT

VALUES OBTAINED FROM JURISDICTION, STATE, AND NATIONAL AGENCIES









BASIC PERMIT PACKAGE  
REVIEWED FOR CODE COMPLIANCE  
WITH IRC 2015  
KITSAAP COUNTY BUILDING DEPARTMENT

08/02/2018 11:56:07 AM  
# 18-02763

SNOW LOAD (UP TO)	NOMINAL THICKNESS	SPAN RATING
40LBS	7/16"	24/16
70LBS	15/32", 1/2"	32/16
130LBS	19/32", 5/8"	40/20
175LBS	23/32", 3/4"	48/24

LONG DIMENSIONS PERPENDICULAR TO  
ROOF JOIST WITH EDGE SUPPORT.  
R909.2.1.1.1

SHEARWALL / ANCHOR SCHEDULE								
LABEL	APA RATED SHEATHING (1) (2) (4) (14) (21)	NAIL SIZE & SPACE @ EDGES (4) (5)	STUD AND BLOCKING SIZE@ ADJOINING EDGES (3) (6) (15) (16)	TO TOP PLATE TO RIM JOIST/ BIRD BLOCK/ BOTTOM CHORD (7) (8)	2X BOTTOM PLATE ATTACHMENT	SILL PLATE ATTACHMENT (NO 3x SILLS REQUIRED)	CAPACITY (PLF) SEISMIC/WIND (OF VALUES) (HF X .93)	
					NAILING TO WOOD BELOW (9)	ANCHOR BOLT TO CONCRETE BELOW (10) (20) (22)		PLACEMENT OF EDGE OF PLATE WASHER (19)
 SW1	7/16" OSB ONE SIDE	8d @ 6" O.C.	2x	16d @ 8" O.C.	16d @ 6" O.C.	5/8" @ 48" O.C. 1/2" @ 36" O.C.	CENTER	260/365
 SW2	7/16" OSB ONE SIDE	8d @ 4" O.C.	3x	LTP4 @ 32" O.C.	16d @ 4" O.C.	5/8" @ 40" O.C. 1/2" @ 28" O.C.	1/2" FROM SHEATHING	380/532
 SW3	7/16" OSB ONE SIDE	8d @ 3" O.C.	3x	LTP4 @ 12" O.C.	(2) ROWS 16d @ 6" O.C.	5/8" @ 24" O.C. 1/2" @ 17" O.C.	1/2" FROM SHEATHING	490/685
 SW4	7/16" OSB ONE SIDE	8d @ 2" O.C.	3x	LTP4 @ 8" O.C.	(2) ROWS 16d @ 4" O.C.	5/8" @ 21" O.C. 1/2" @ 15" O.C.	1/2" FROM SHEATHING	640/895
 SW5	7/16" OR 5/8" GWB	8d @ 7" O.C.	2x	6d COOLER @ 4" O.C.	16d @ 8" O.C.	5/8" @ 60" O.C. 5/8" @ 60" O.C.	CENTER	145/145
 SW6	7/16" OSB BOTH SIDES	8d @ 3" O.C.	3x	LTP4 @ 6" O.C.	16d @ 4" O.C. & LTP5 @ 12" O.C.	5/8" @ 12" O.C. 1/2" @ 8" O.C.	4-1/2" WASHER REQUIRED	980/1370

SHEARWALL NOTES:

- (1) INSTALL PANELS VERTICALLY - DO NOT BREAK AT RIM UNLESS STRUCTURAL RIM JOIST.
- (2) WHERE SHEATHING IS APPLIED ON BOTH SIDES OF WALL, PANEL EDGE JOINTS ON 2X FRAMING SHALL BE STAGGERED SO JOINTS ON THE OPPOSITE SIDES ARE NOT LOCATED ON THE SAME STUDS.
- (3) BLOCKING IS REQUIRED AT ALL PANEL EDGES.
- (4) PROVIDE SHEARWALL SHEATHING AND NAILING FOR THE ENTIRE LENGTH OF THE WALLS INDICATED ON THE PLANS. ENDS OF FULL HEIGHT WALLS ARE DESIGNATED BY EXTERIOR OF THE BUILDING, CORRIDORS, WINDOWS, OR DOORWAYS OR AS DESIGNATED ON THE PLANS. SEE PLANS FOR HOLDOWN REQUIREMENTS.
- (5) SHEATHING EDGE NAILING IS REQUIRED AT ALL HOLDOWN POSTS.
- (6) INTERMEDIATE FRAMING TO BE WITH 2X MINIMUM MEMBERS. FIELD NAILING IS 12" O.C. UNLESS NOTED OTHERWISE.
- (7) BASED ON 0.131" x 1.1/2" LONG NAILS USED TO ATTACH FRAMING CLIPS DIRECTLY TO FRAMING. USE 0.131" x 2 1/2" LONG NAILS WHEN NAILING THROUGH SHEATHING. USE A35 OR RBC CLIPS IN LIEU OF LTPs FOR ROOF BLOCKING TO TOP PLATE (SPACE AT 48" O.C. FOR SW1 SHEARWALLS).
- (8) 16d @ 4" O.C. TYP. UNLESS NOTED OTHERWISE.
- (9) WHERE PLATE ATTACHMENTS SPECIFY (2) ROWS OF NAILS, PROVIDE DOUBLE JOIST, RIM OR EQUAL ATTACH PER DETAILS.
- (10) ANCHOR BOLTS SHALL BE PROVIDED WITH STEEL PLATE WASHERS 1/4" x 3" x 3", EMBED ANCHORS A MINIMUM OF 7" INTO THE CONCRETE. MAX 1/2" FROM SHEATHING SIDE.
- (11) PRESSURE TREATED MATERIALS CAN CAUSE EXCESSIVE CORROSION IN THE FASTENERS. PROVIDE HOT-DIPPED GALVANIZED (ELECTRO PLATING NOT PERMITTED) NAILS AND CONNECTOR PLATES (FRAMING ANGLES, ETC.) FOR ALL CONNECTORS IN CONTACT WITH PRESSURE TREATED FRAMING MEMBERS.
- (12) NOT USED.
- (13) 3/8" PLYWOOD SHEATHING MAY BE USED IN PLACE OF THE 1/4" OSB SHEATHING PROVIDED THAT ALL STUD SPACING IS 16" O.C. MAX.
- (14) WHERE WOOD SHEATHING (W) IS APPLIED OVER GYPSUM SHEATHING (G) CONTACT THE ENGINEER OF RECORD FOR ALTERNATE NAILING REQUIREMENTS.
- (15) AT ADJOINING PANEL EDGES, (2) 2X STUDS NAILED TOGETHER MAY BE USED TOGETHER IN PLACE OF A SINGLE 3X STUD. DOUBLE 2X STUDS MAY BE CONNECTED TOGETHER BY NAILING WITH 3" LONG NAILS SPACED CONSISTENTLY WITH PLATE NAILING.
- (16) CONTACT THE ENGINEER OF RECORD FOR ADHESIVE OR EXPANSION BOLT ALTERNATIVES TO CAST IN PLACE ANCHOR BOLTS. (INSPECTION MAY BE REQUIRED).
- (17) NOT USED.
- (18) NOT USED.
- (19) EDGE OF PLATE WASHER A MAXIMUM OF 1/2" FROM THE SHEATHED SIDE OF THE SHEAR WALL.
- (20) USE 5/8" GWB FOR SWS WHERE SPECIFIED BY DESIGNER.
- (21) SHEARWALL DESIGNATED WITH AN ASTERISK (\*) MAY BE SHEATHED WITH 7/16 SMART PANEL OR 5/16 HARDIE PANEL.
- (22) SIMPSON MASAP (STANDARD INSTALLATION) MAY BE SUBSTITUTED FOR ANCHOR BOLT. USE 1/2" AB SPACING

HOLDOWN SCHEDULE			
SIMPSON PRODUCT MODEL NUMBER (1,2,3) CAPACITY#	FASTENERS		ANCHOR BOLTS
	NAILS	SCREWS	
HDU5 - HF MEMBER 4065# HDU5 - DF MEMBER 5645#	N/A	(14)SDS 1/4" x 2 1/2" TO (2) STUDS MIN.	SB 5/8x24 U.N.O.
HDU8 - HF MEMBER 5655# HDU8 - DF MEMBER 7870#	N/A	(20)SDS 1/4" x 2 1/2" TO (2) STUDS MIN.	SSTB28 OR SB 7/8"x24 U.N.O.
HDU11 - HF MEMBER 6865# HDU11 - DF MEMBER 9535#	N/A	(30)SDS 1/4" x 2 1/2" TO 6X6 MIN.	SB1x30" U.N.O.
HDU14 - HF MEMBER 10350# HDU14 - DF MEMBER 14445#	N/A	(36)SDS 1/4" x 2 1/2" TO 6X6 MIN.	SB1x30" U.N.O.
STHD14/STHD14RJ - 3500#	(2) STUDS W/ (30) 16d	N/A	N/A
MSTC48B3 - 3420#	(2) STUDS W/ (20) 10d	N/A	N/A
MSTC66B3 - 3875#	(2) STUDS W/ (20) 10d	N/A	N/A
CS14 - 2490#	(18) 8d EACH SIDE	N/A	N/A

HOLDOWN NOTES:

- (1) INSTALL ALL SHEAR ANCHORS PER MANUFACTURER INSTRUCTIONS.
- (2) ALL METAL CONNECTORS COMING IN CONTACT WITH PRESSURE TREATED WOOD SHALL BE SIMPSON "Z-MAX", TRIPLE ZINC COATED, OR HOT DIPPED GALVANIZED FOR CORROSION PROTECTION.
- (3) SUBSTITUTION WITH NON-SIMPSON ANCHORS WITH ENGINEER'S APPROVAL.
- (4) STHD14RJ INSTALLATION AT RIM JOIST LOCATIONS ONLY (RJ-RIM JOIST)

MUST COMPLY WITH ALL  
WASHINGTON STATE CODES

08/02/2018 11:56:23 AM  
# 18-02763

Validity of permit: This issuance or granting of a permit shall not be  
considered to be a permit for, or an approval of, any violation of any of  
the provisions of the International Codes or any other ordinance of  
Kitsap County. Permittees desiring to give authority to violate or  
cancel the provisions of the International Codes and ordinances of  
Kitsap County shall not be valid.

08/02/2018 11:56:32 AM  
# 18-02763

LEGEND

POINT LOAD KEY  
FOR ENGINEER USE ONLY  
ROUNDED TO NEAREST KIP

MEMBER DEAD LIVE SNOW  
TOTAL  
1.0 2.0 3.0  
1.0 3.0 3.0

BEAM DESIGNATOR  
FOR PLAN REVIEW USE ONLY

STUD/COLUMN  
DESIGNATOR

SHEARWALL DESIGNATOR  
SEE SHEARWALL SCHEDULE

HOLDOWN/STRAP  
DESIGNATOR  
SEE HOLDOWN/STRAP SCHEDULE

DETAIL CALL-OUT  
SEE TOP SHEETS FOR  
STRUCTURAL DETAILS

1 D1

TALL CRAWL SPACE  
LATERAL ENGINEERING  
DO NOT SCALE (1/4"=1')

SHEATH ALL EXTERIOR WALLS AS SW1 UNLESS NOTED OTHERWISE.  
ADJUST CRAWL SPACE VENTS TO BE 12" MINIMUM AWAY FROM HOLDOWNS

MAIN FLOOR LATERAL ENGINEERING

DO NOT SCALE (1/4"=1')

SHEATH ALL EXTERIOR WALLS AS SW1 UNLESS NOTED OTHERWISE.  
ADJUST CRAWL SPACE VENTS TO BE 12" MINIMUM AWAY FROM HOLDOWNS

3" O.C. NAILING - SHEATHING  
TO MUD SILL TYPICAL ALL  
EXTERIOR WALLS

PROVIDE ALL FINAL SHOP DRAWINGS TO HODGE  
ENGINEERING AS SOON AS COMPLETED.  
IF DESIGNER'S DRAWINGS DIFFER STRUCTURALLY FROM  
SHOPS ADDITIONAL ENGINEERING WILL BE REQUIRED.  
INSTALL AND BRACE JOISTS AND TRUSSES PER  
MANUFACTURER

UPPER FLOOR LATERAL ENGINEERING

DO NOT SCALE (1/4"=1')

SHEATH ALL EXTERIOR WALLS AS SW1 UNLESS NOTED OTHERWISE.

CONTRACTED FOR LATERAL ENGINEERING  
ONLY, ALL OTHER ENGINEERING INCLUDING  
GRAVITY LOAD ENGINEERING (BEAMS,  
FOUNDATION, ETC.) BY DESIGN  
PROFESSIONAL IN RESPONSIBLE CHARGE  
(IBC 107.3.4)

REVISIONS	DATE	NO.

DO NOT SCALE  
Any variations from conditions  
and dimensions shown on the  
plans shall be the responsibility  
of the designer and engineer for  
the project. The contractor shall  
be responsible for the costs  
of any necessary  
revisions.

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**Hodge** inc.  
engineering

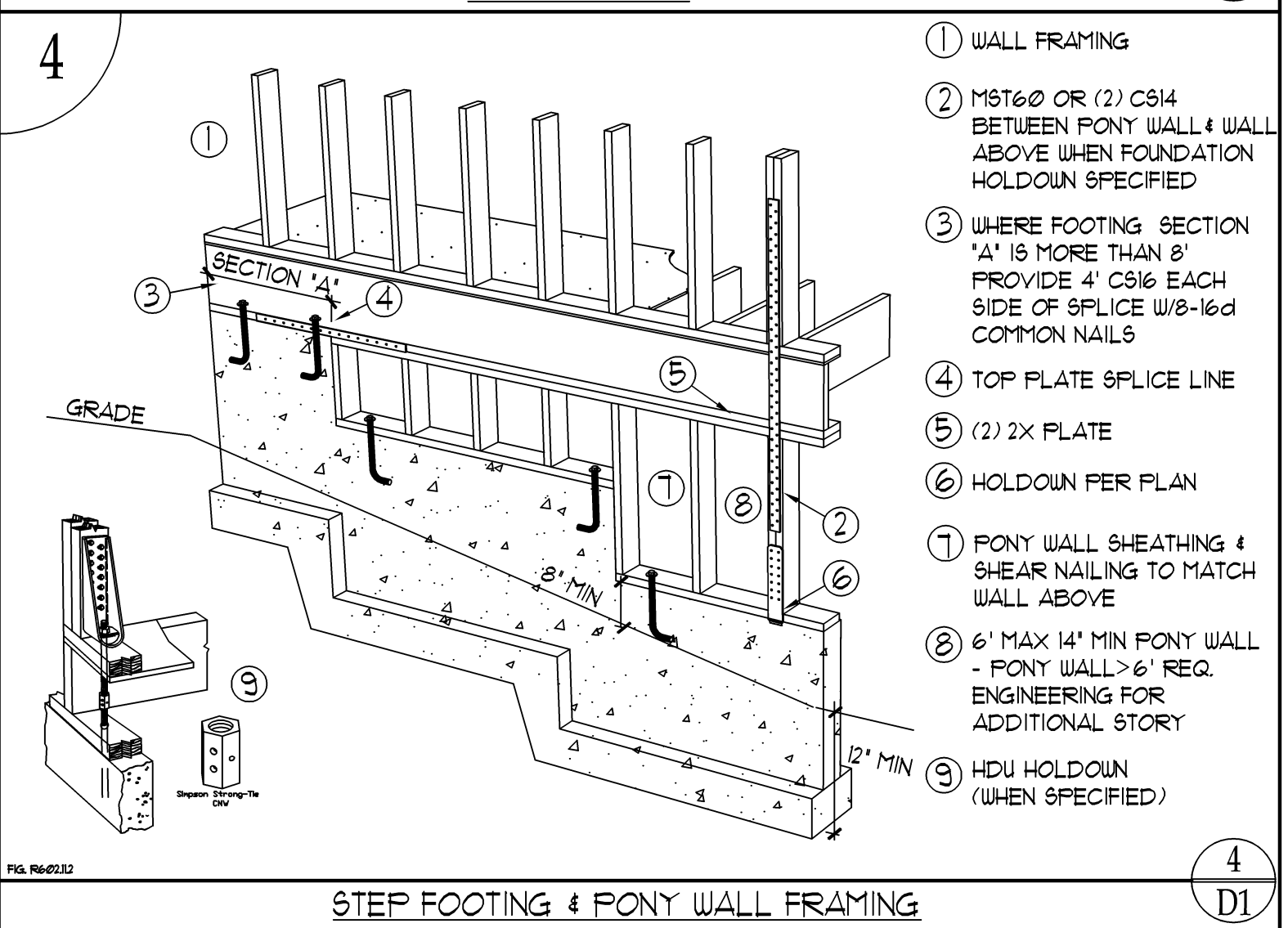
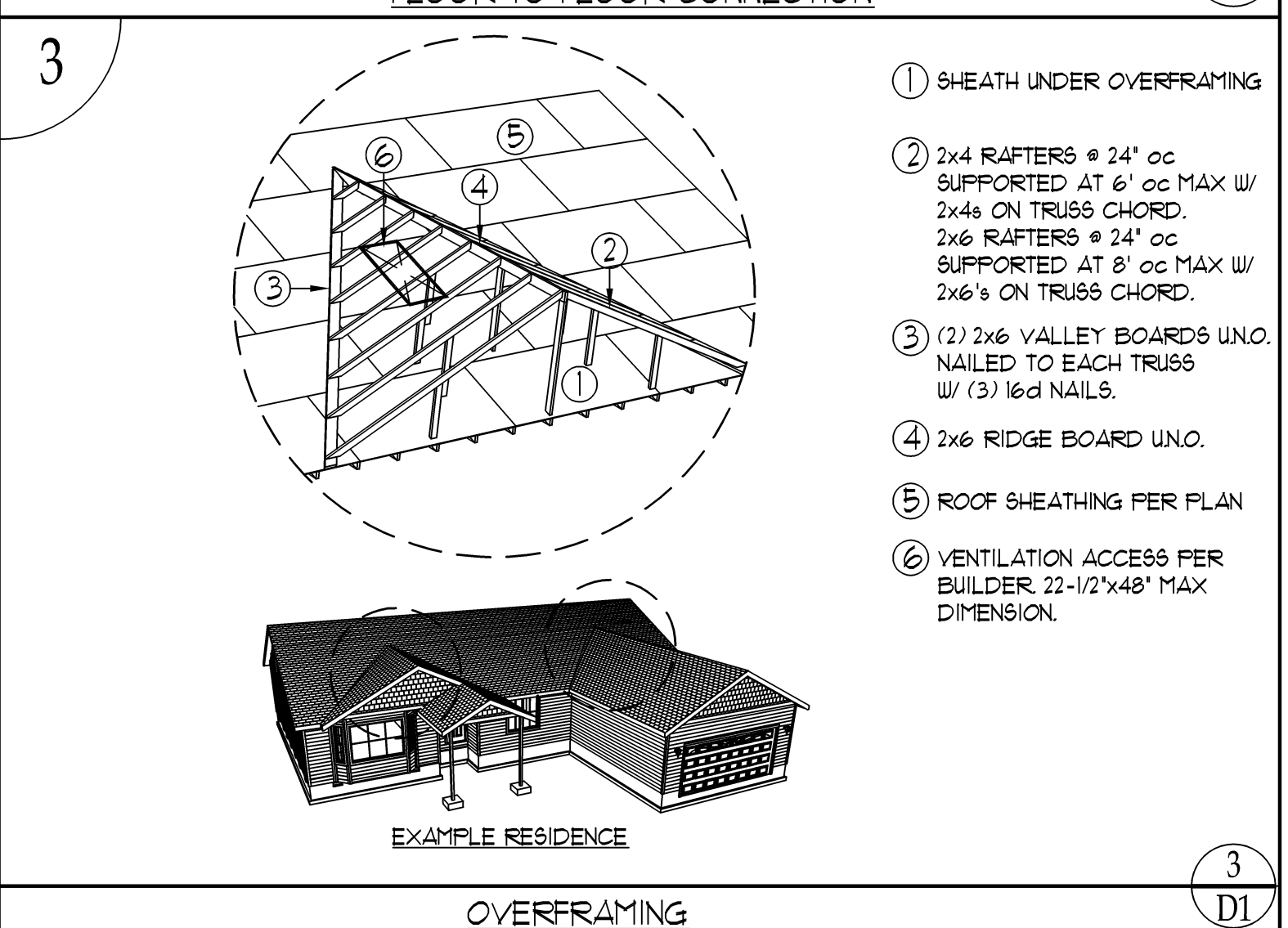
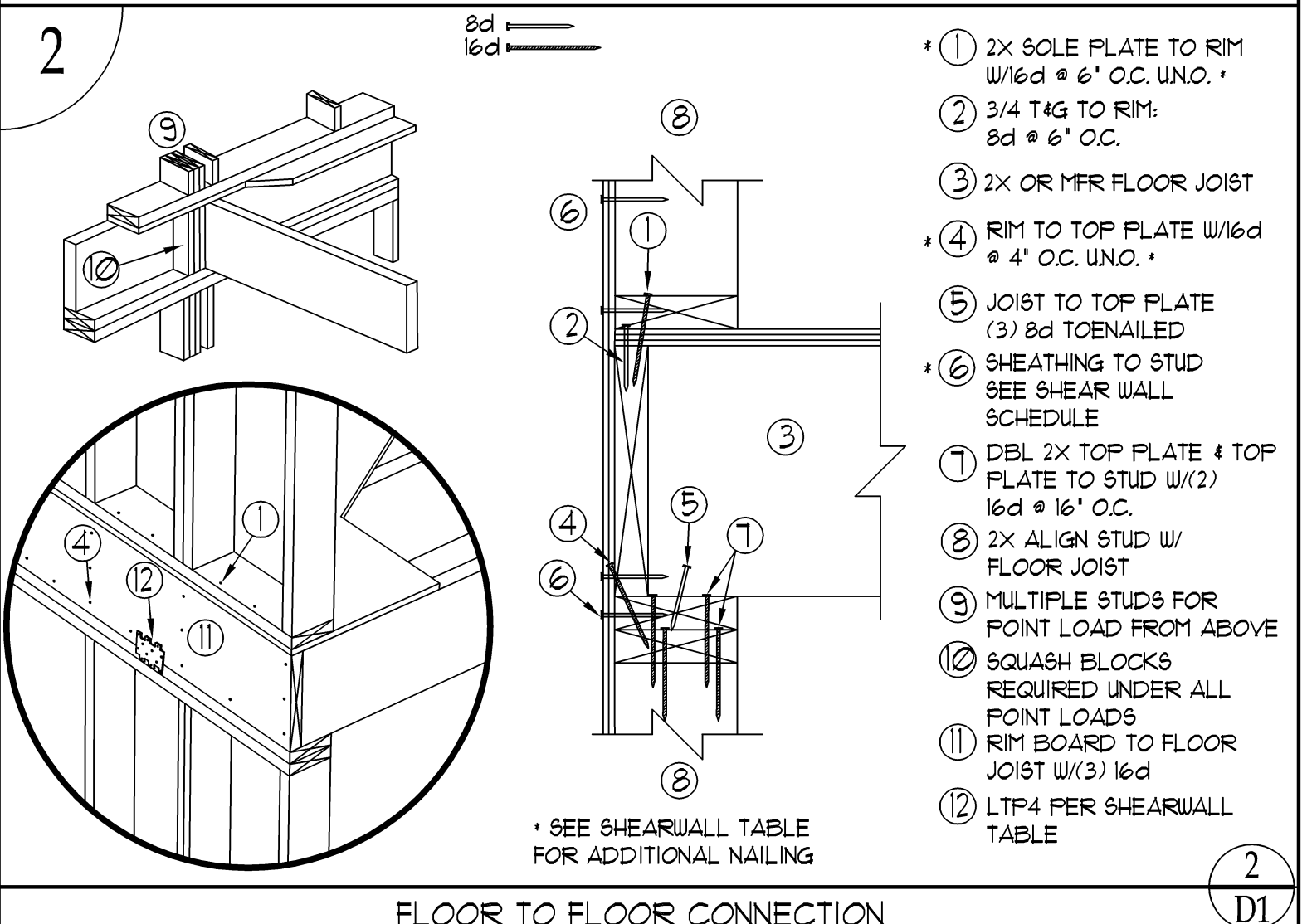
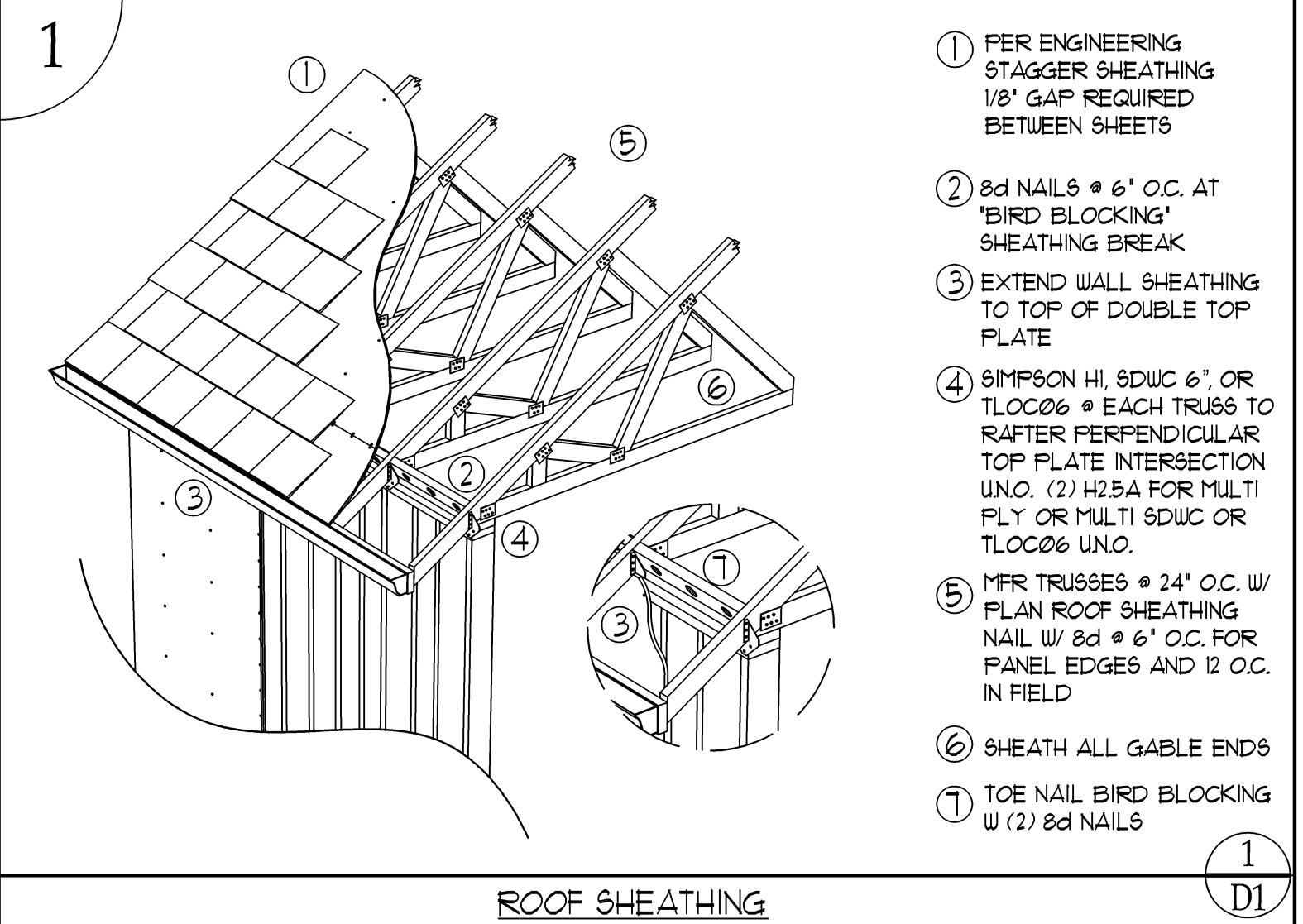
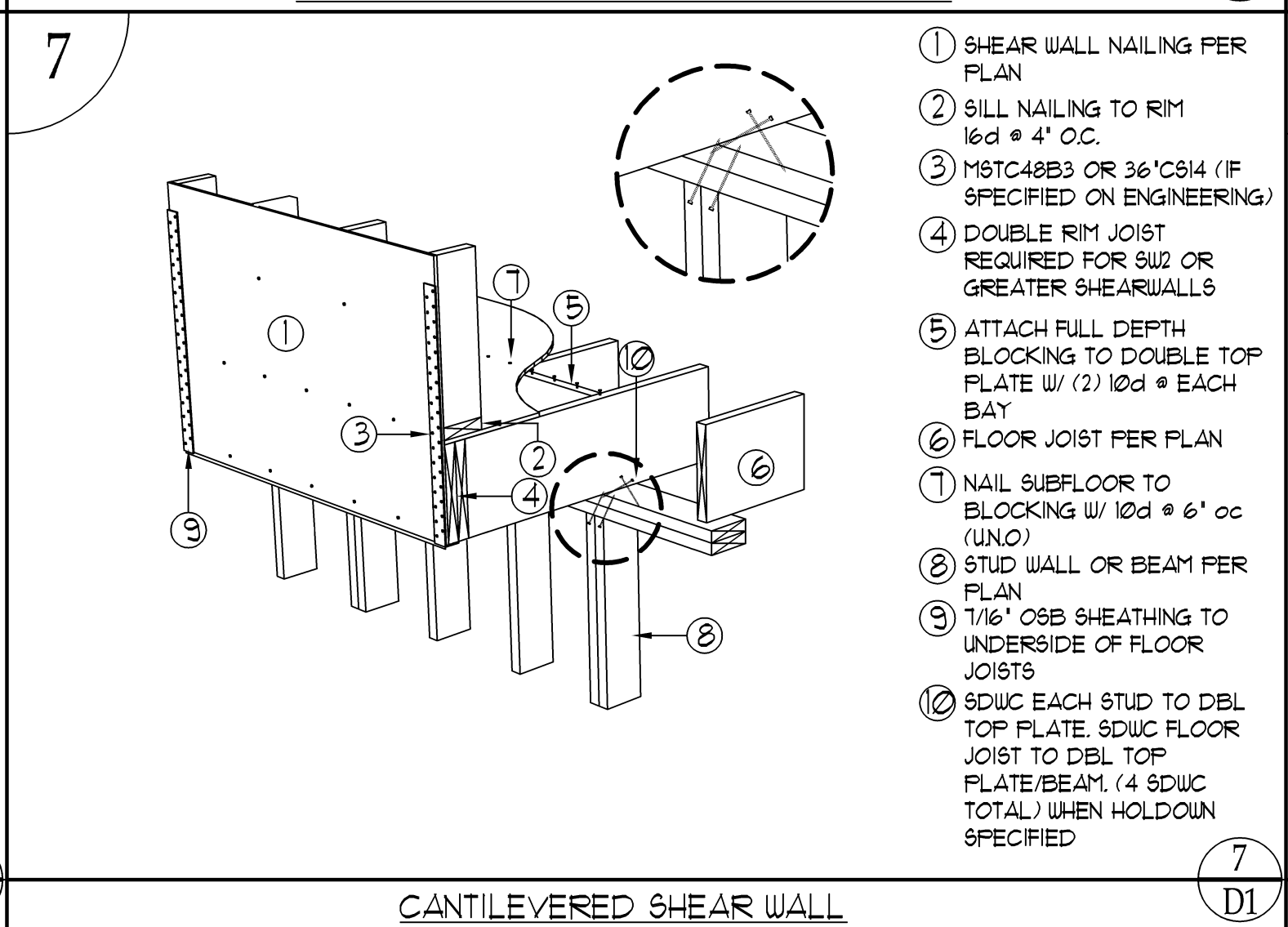
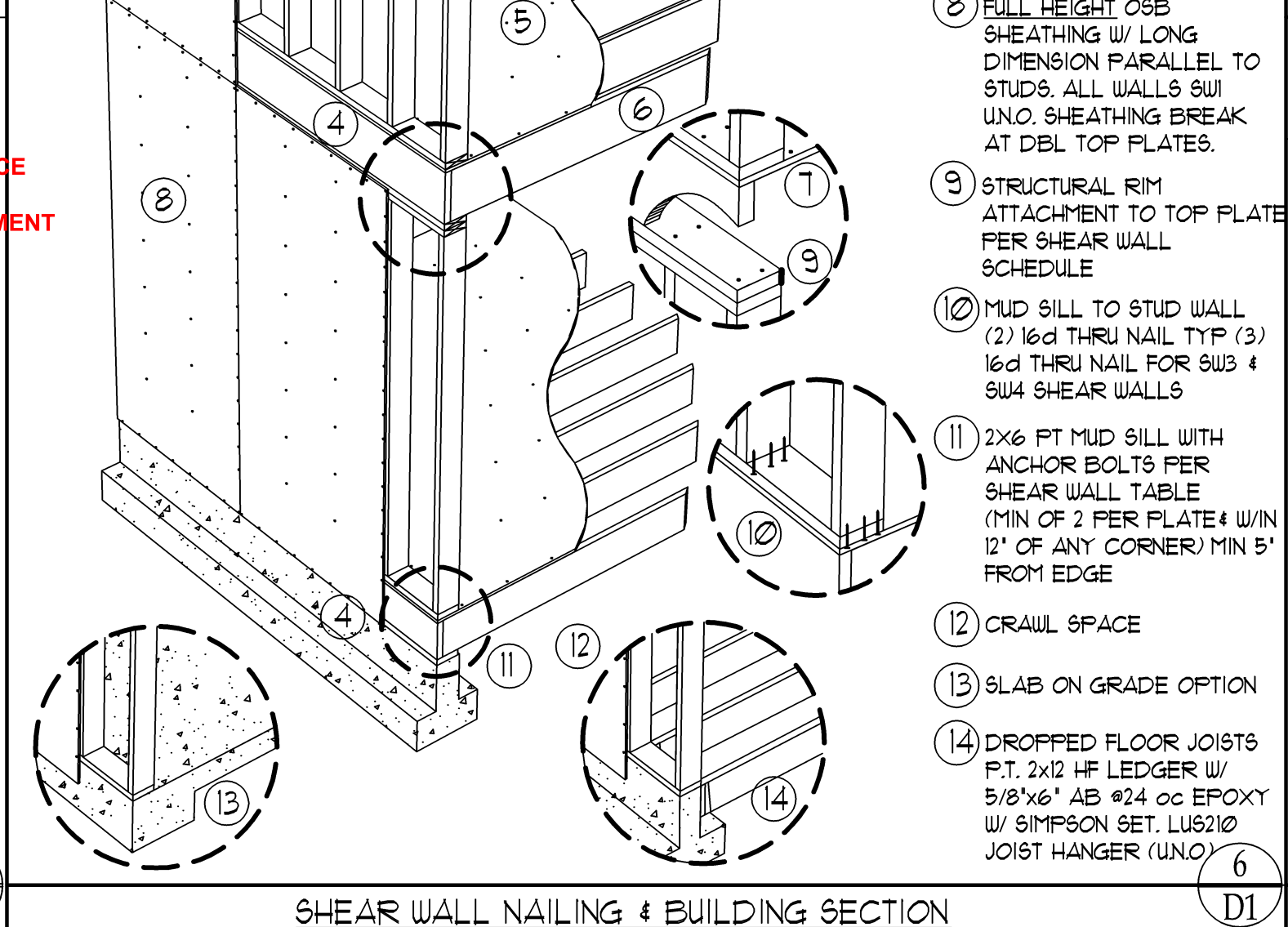
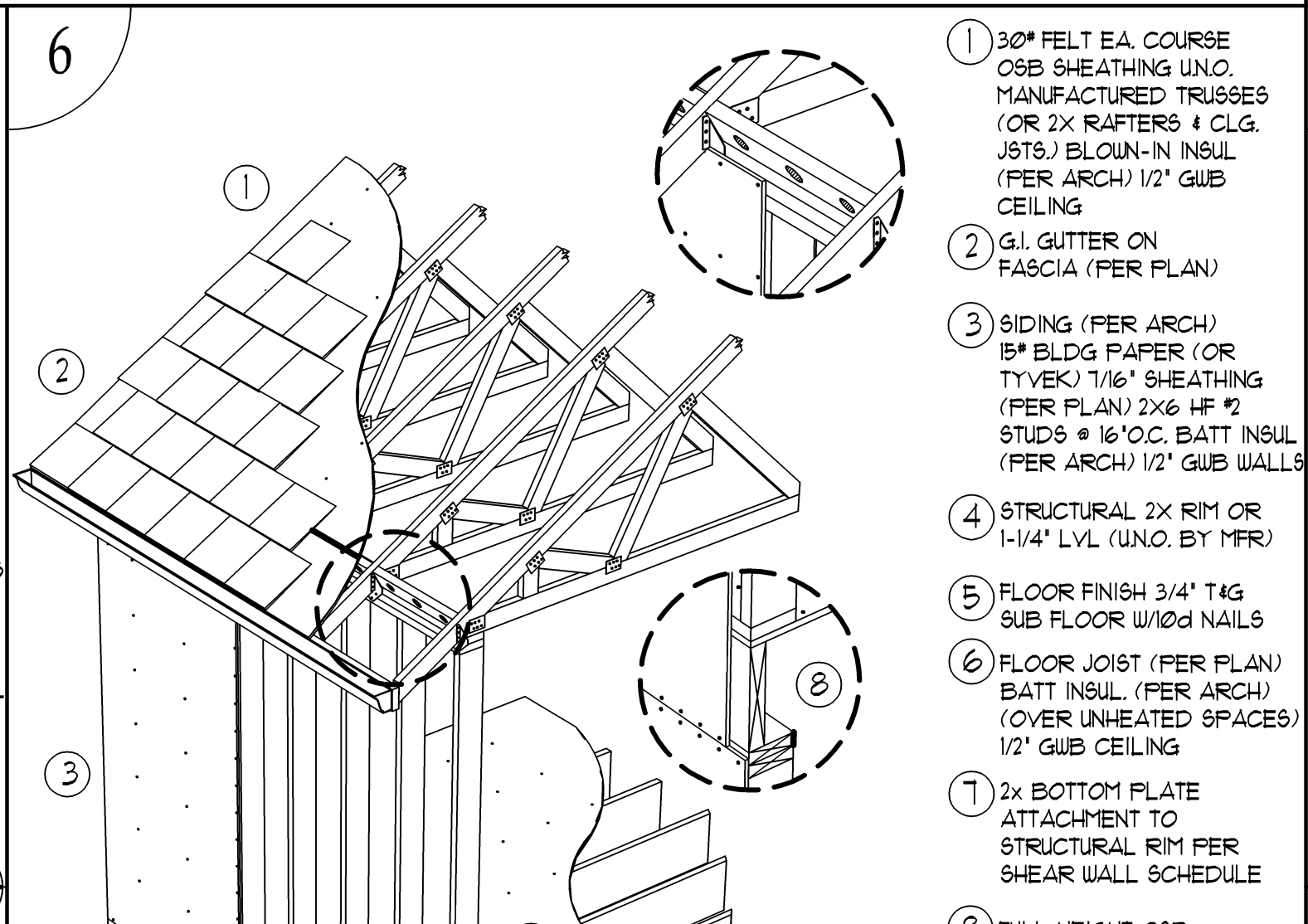
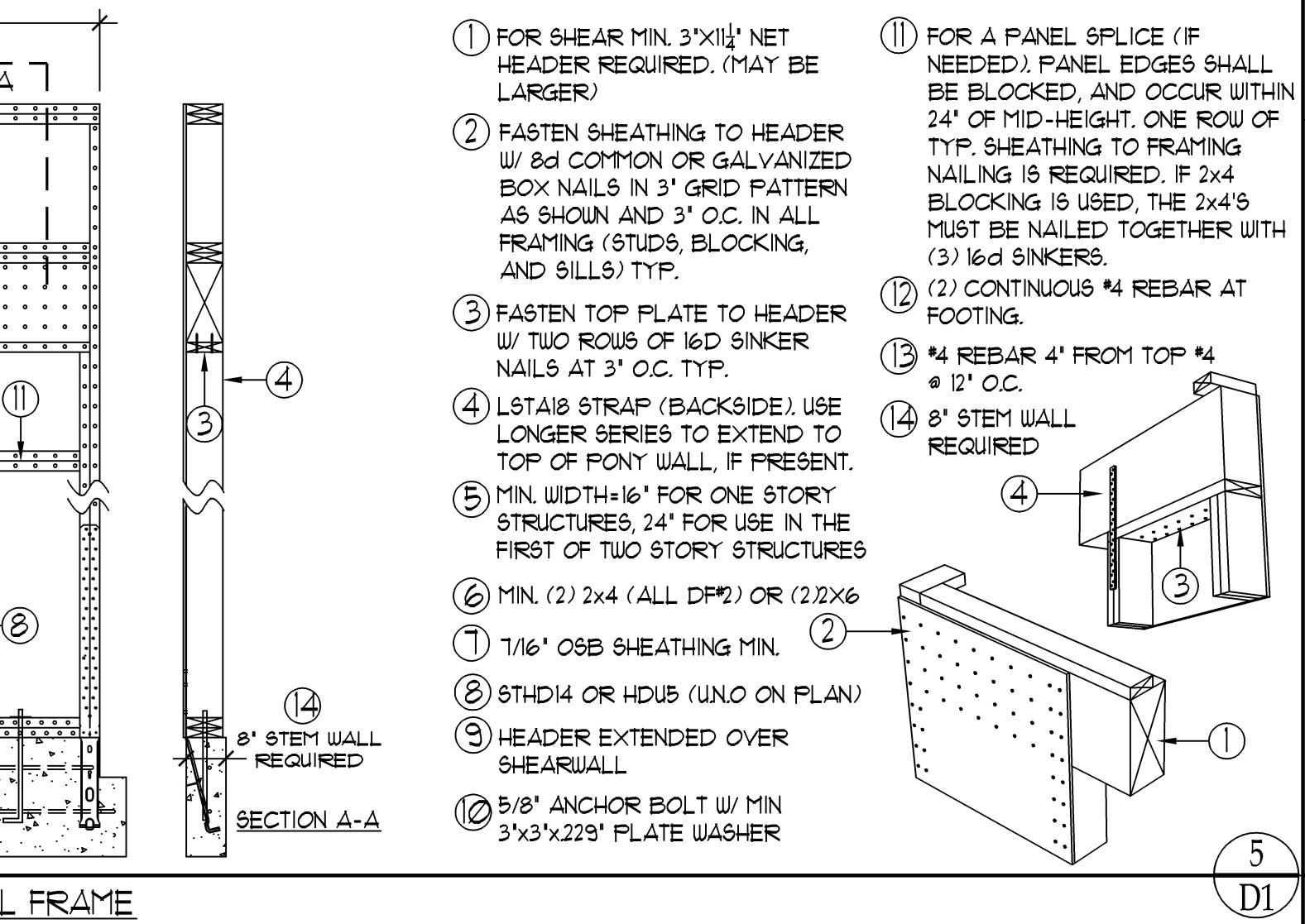
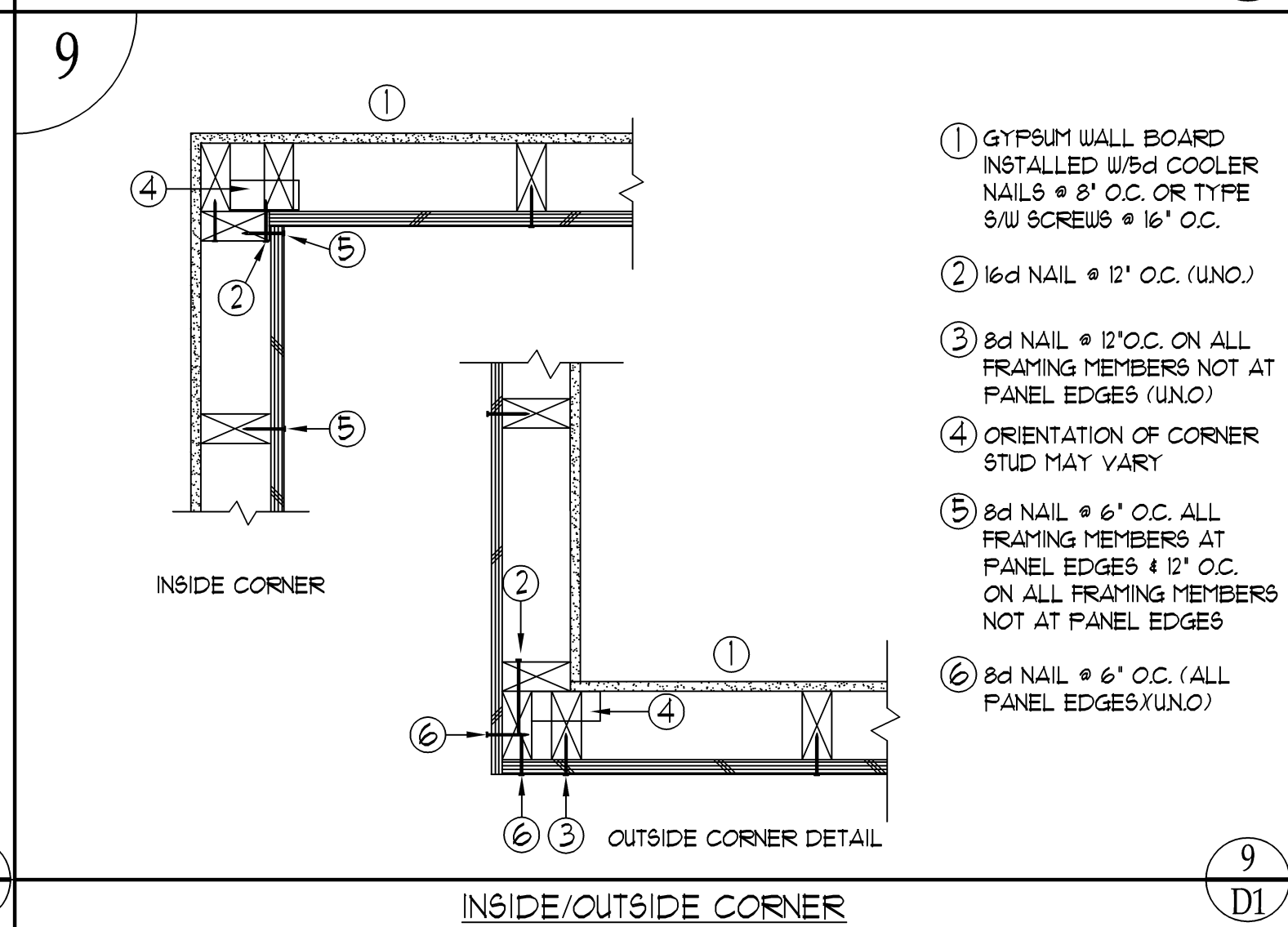
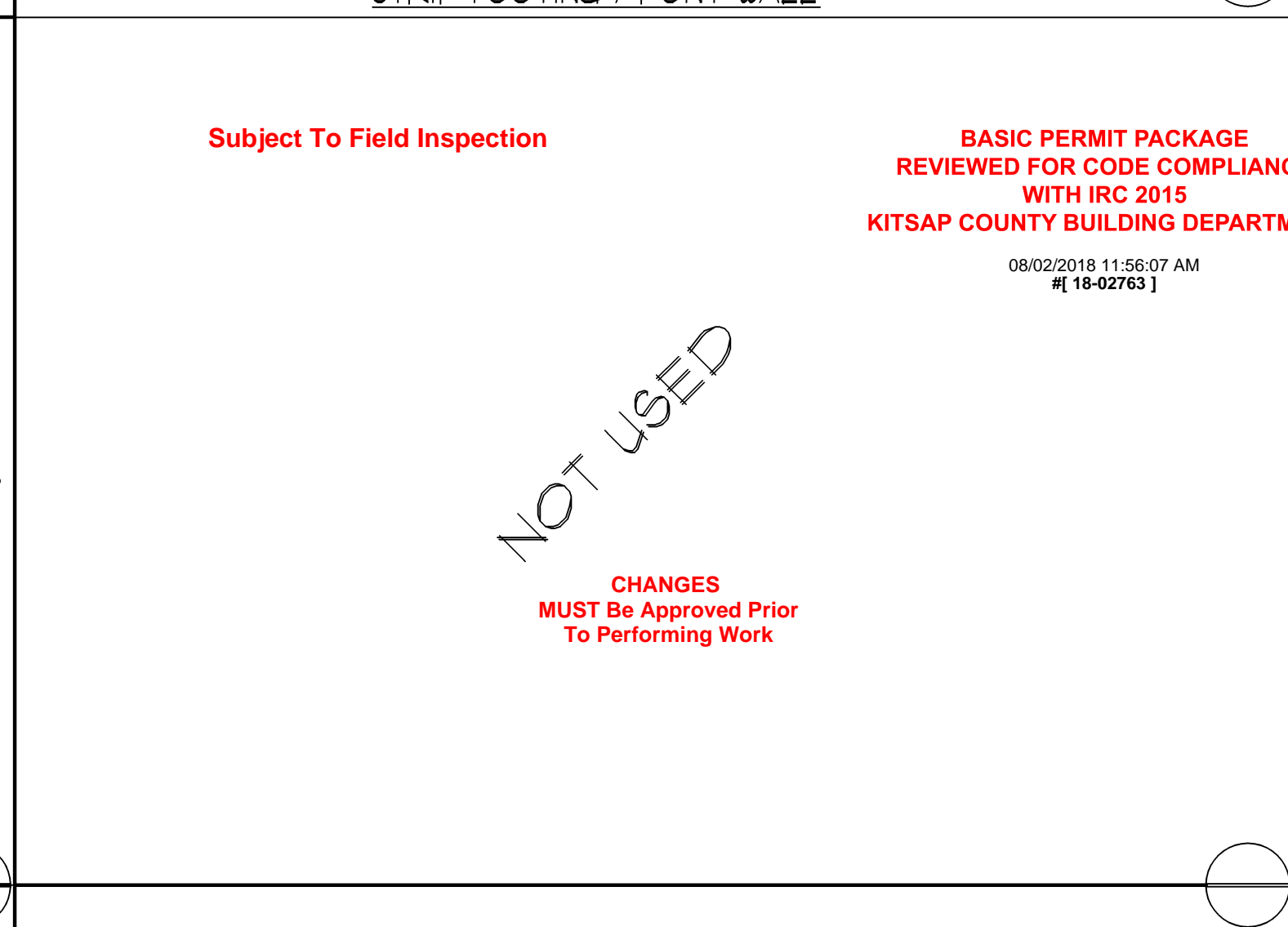
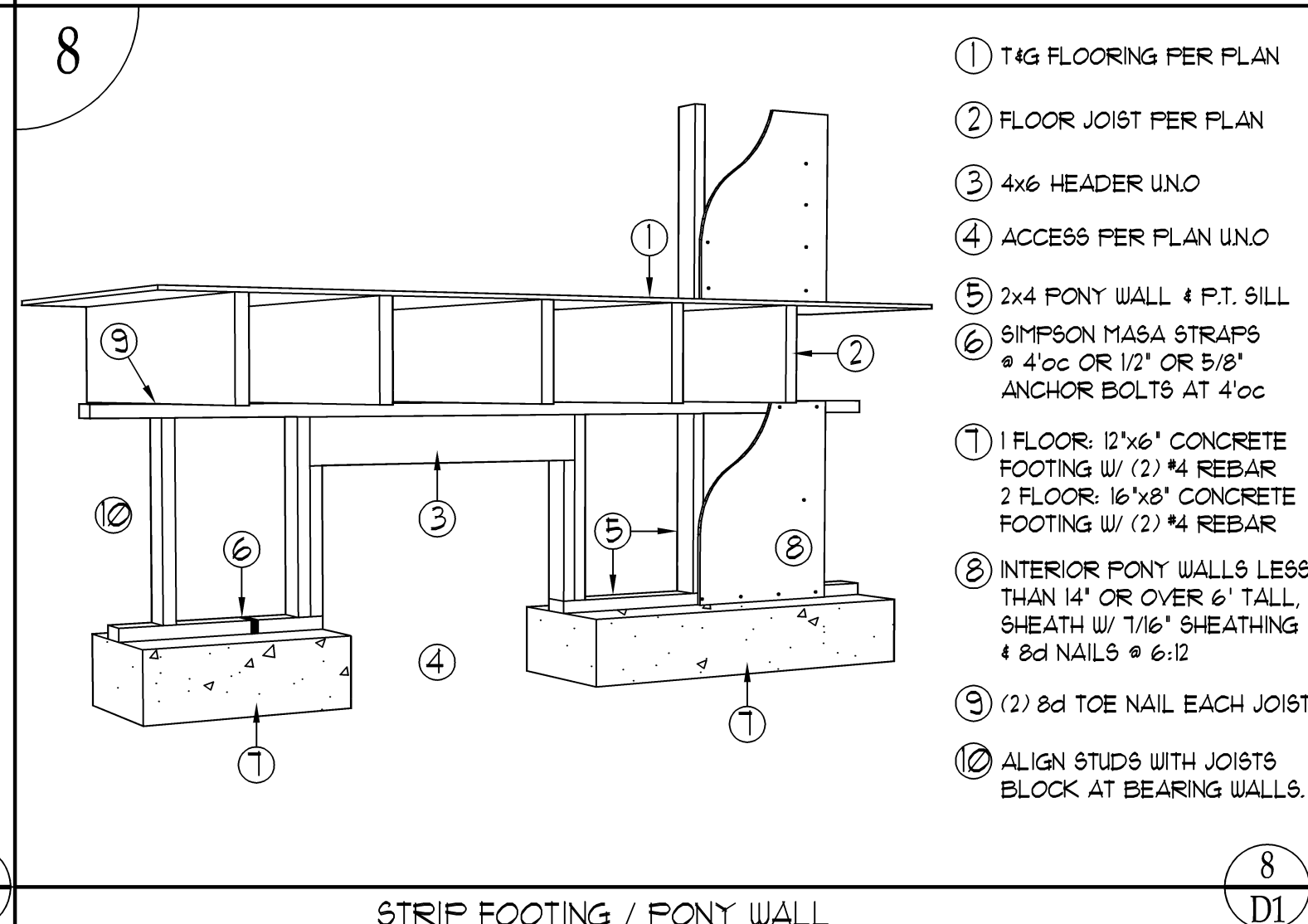
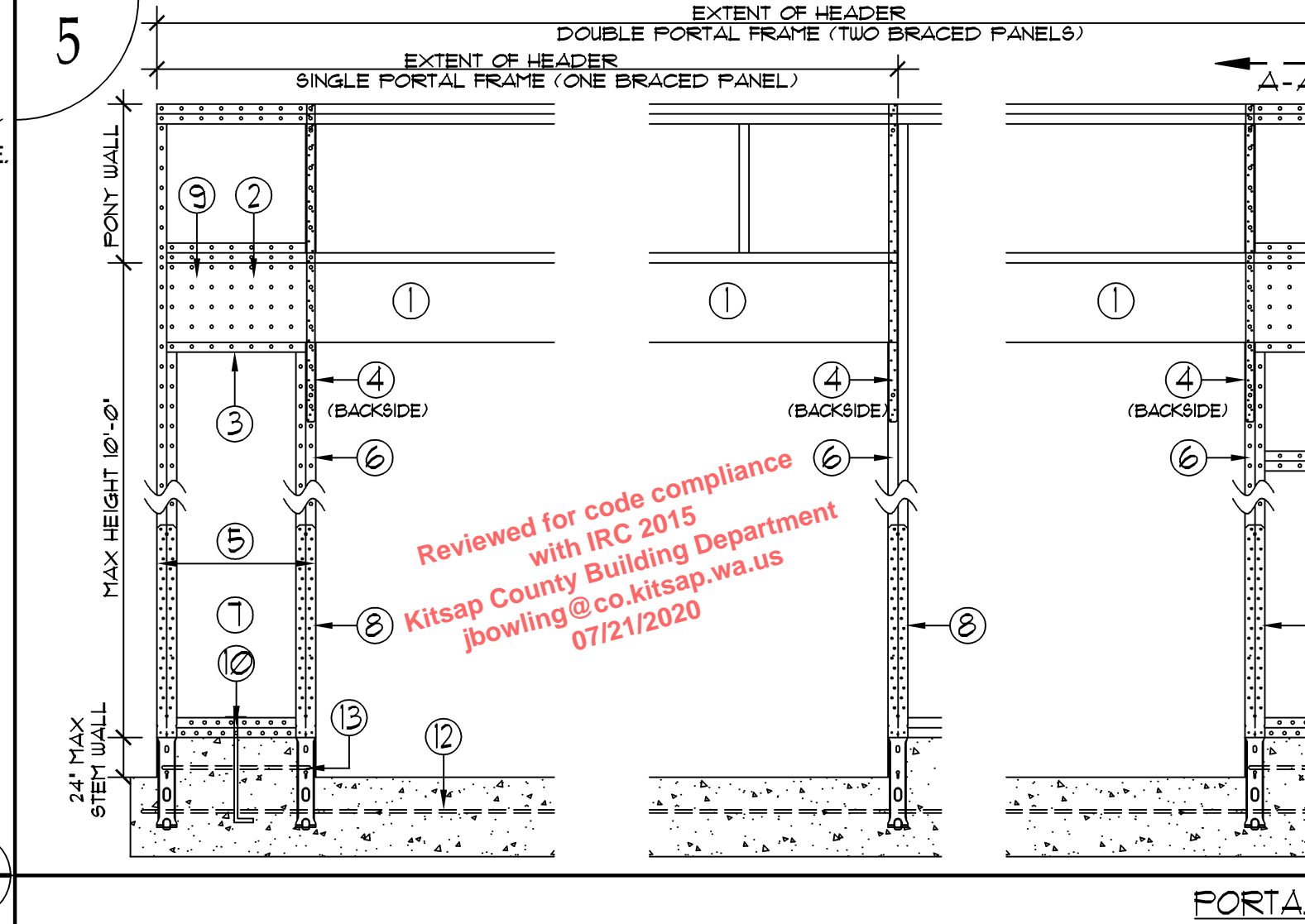
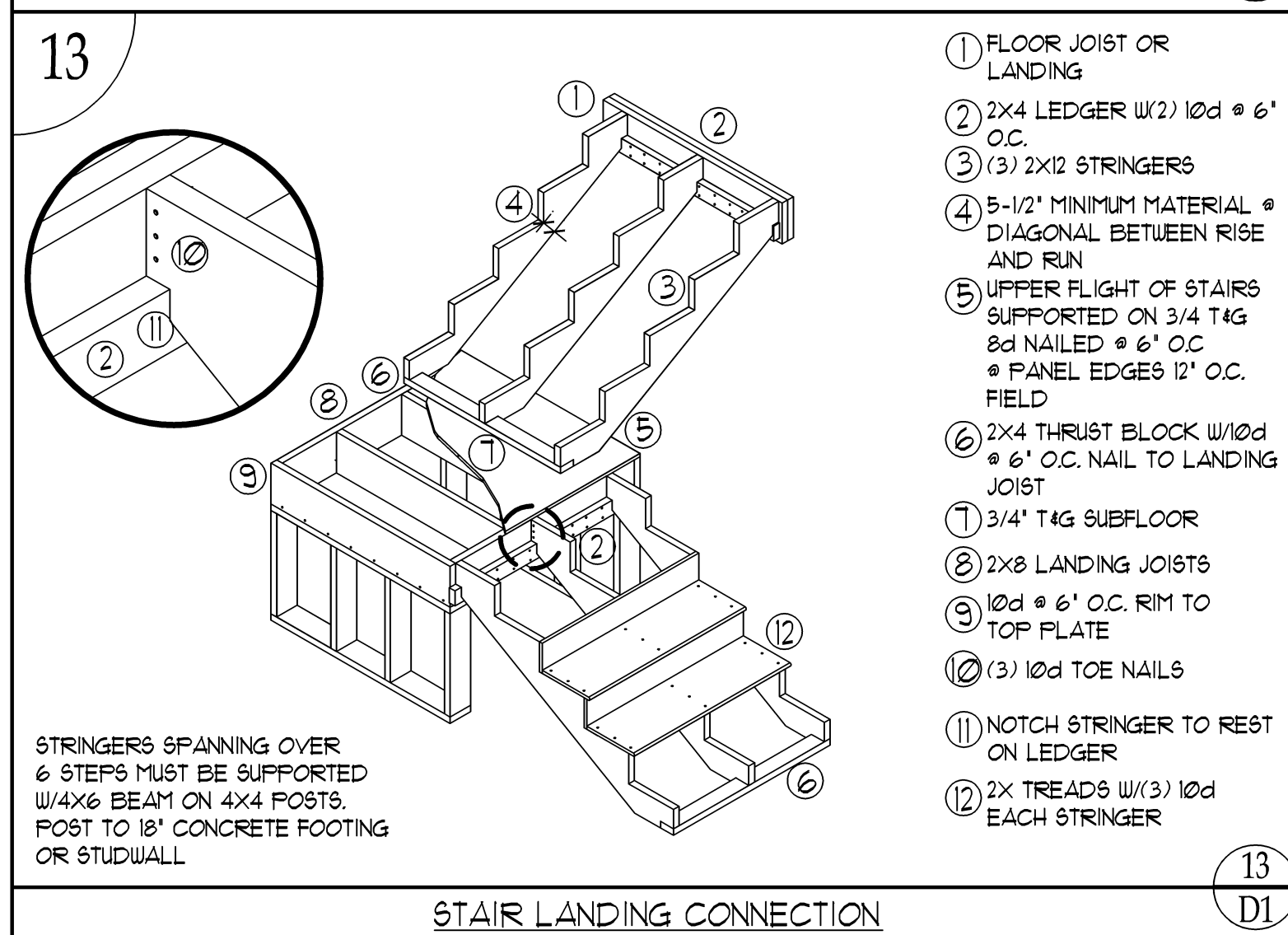
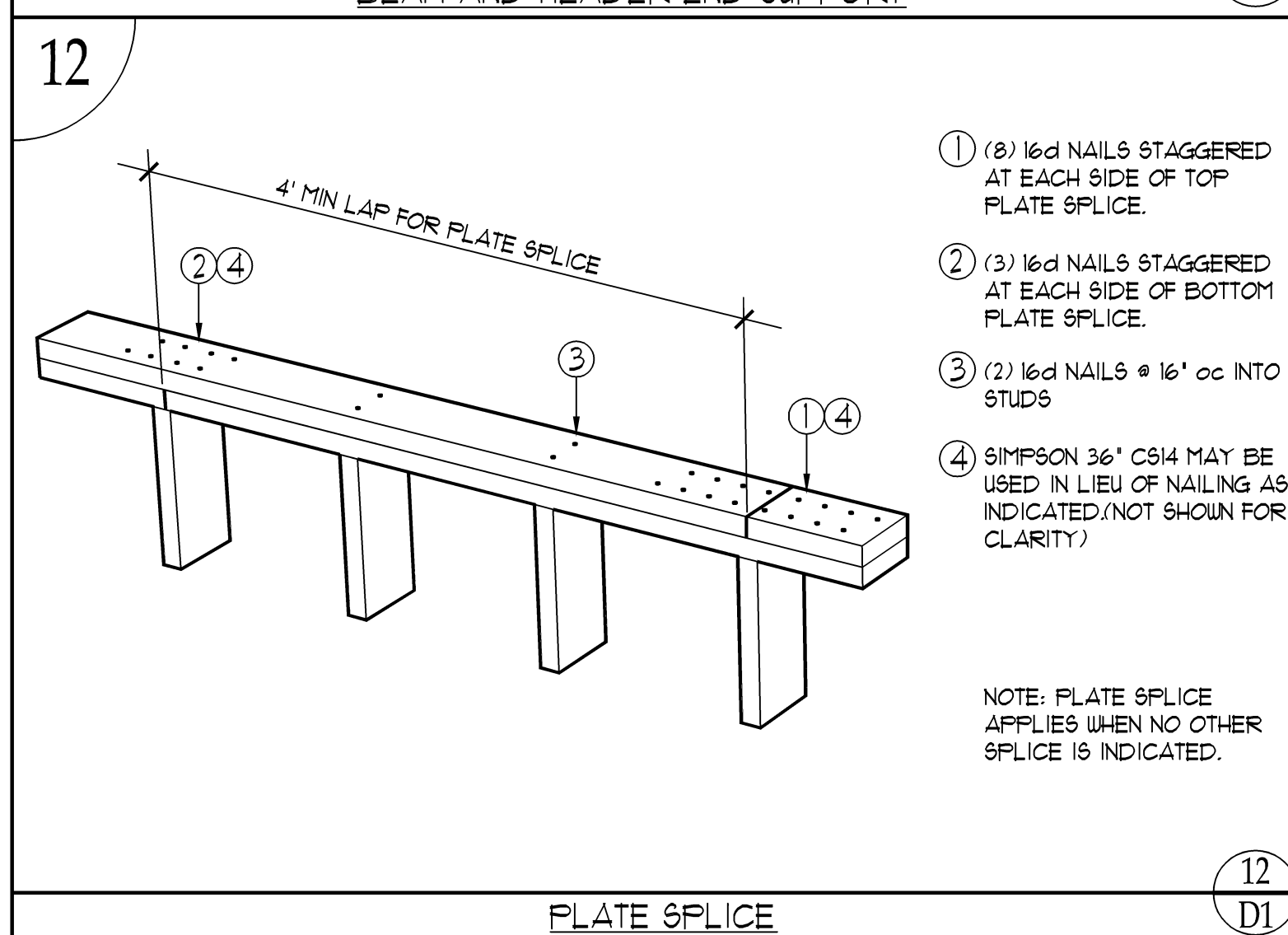
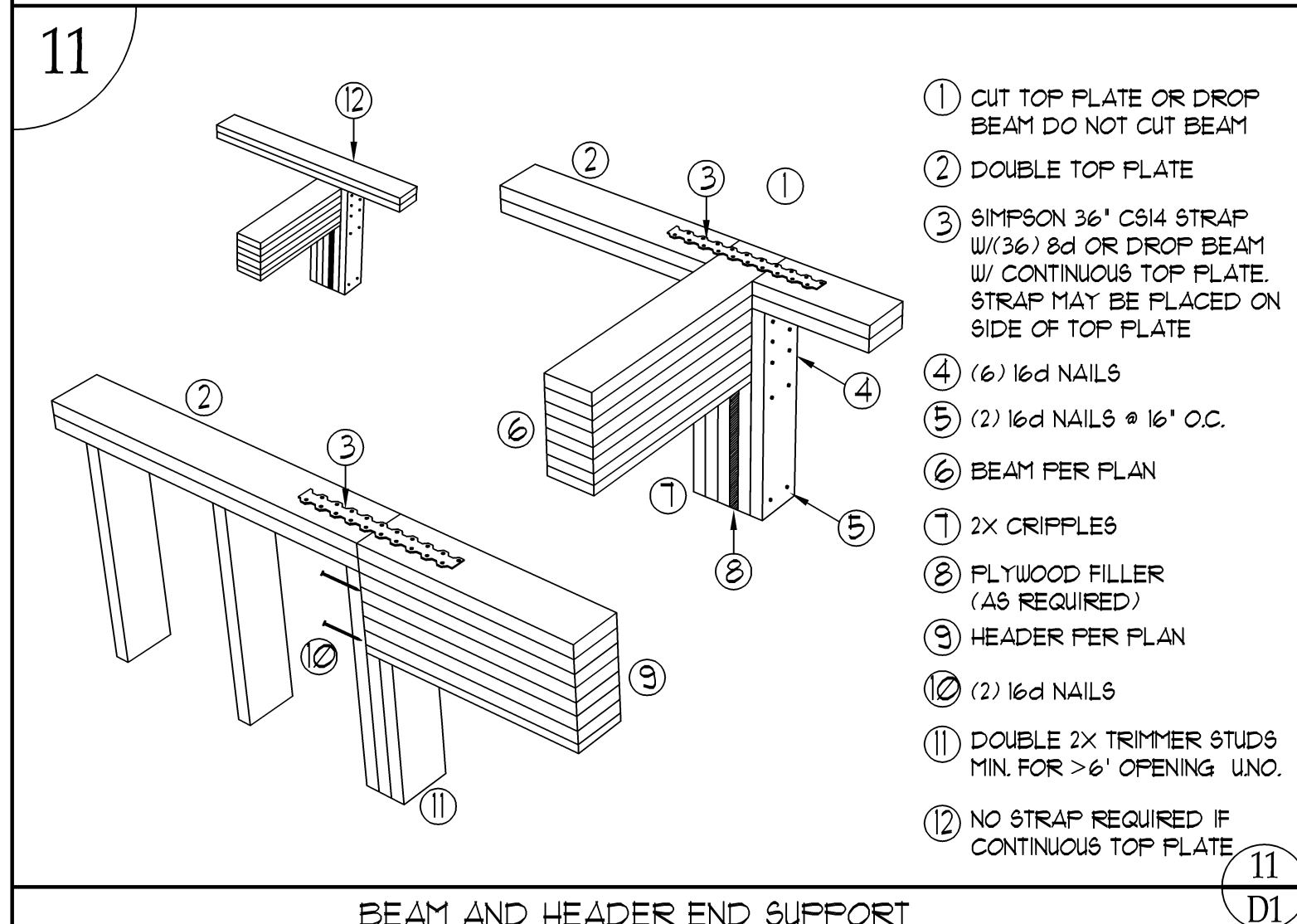
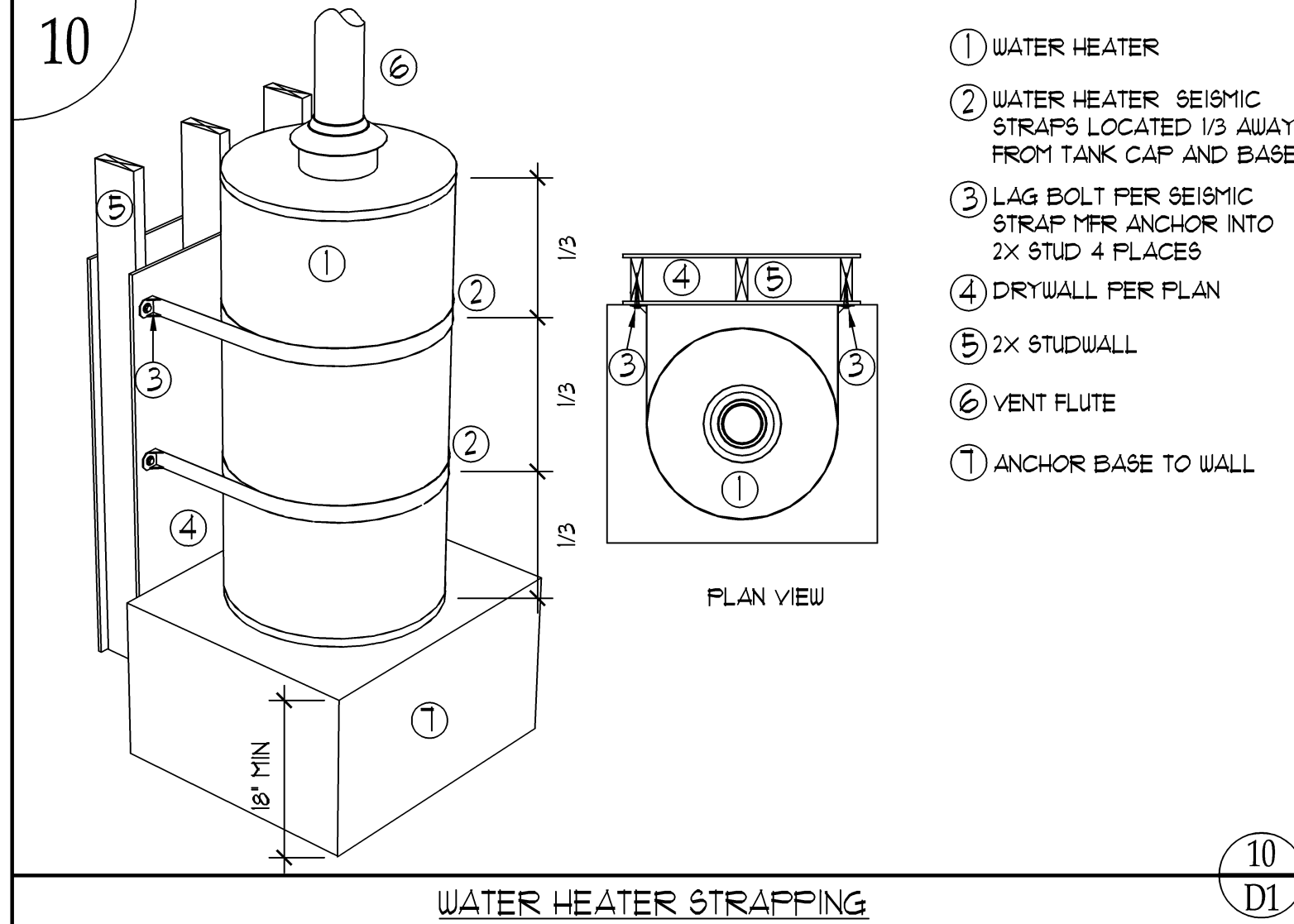
FERGUSON & COLE, INC  
2800  
SITE SPECIFIC LETTER  
REQUIRED FOR EACH USE

BUILDING  
CODE 2015 IBC  
WIND  
EXP. B 110 MPH  
SEISMIC DESIGN  
CATEGORY D  
ROOF SNOW  
LOAD 25 PSF

ENG. P.E. JOHN H.  
DATE 05/31/18  
REVIEW ZACK C.  
ANALYST ANNA N.  
SHEET  
**L1**  
1 OF 4 SHEETS  
PROJECT NUMBER  
180558

06/04/2018  
REGISTERED  
PROFESSIONAL ENGINEER  
COPYRIGHT © 2018  
HODGE ENGINEERING, INC.





REVISIONS

NO.	DATE	DESCRIPTION

DO NOT SCALE

Any variations from conditions and dimensions shown on the drawings shall be the responsibility of the designer and engineer for the drawings. The contractor shall be responsible for the accuracy of the drawings and the contractor shall be responsible for the accuracy of the drawings and the contractor shall be responsible for the accuracy of the drawings.

**Hodge** engineering inc.

**FERGUSON & COLE, INC**  
2800  
SITE SPECIFIC LETTER  
REQUIRED FOR EACH USE

BUILDING CODE 2015 IBC  
WIND EXP. B 110 MPH  
SEISMIC DESIGN CATEGORY D  
ROOF SNOW LOAD 25 PSF

ENG. P.E. JOHN H.  
DATE 05/31/18  
REVIEW ZACK C.  
ANALYST ANNA N.  
SHEET **D1**  
2 OF 4 SHEETS  
PROJECT NUMBER 180558

06/04/2018  
PROFESSIONAL ENGINEER  
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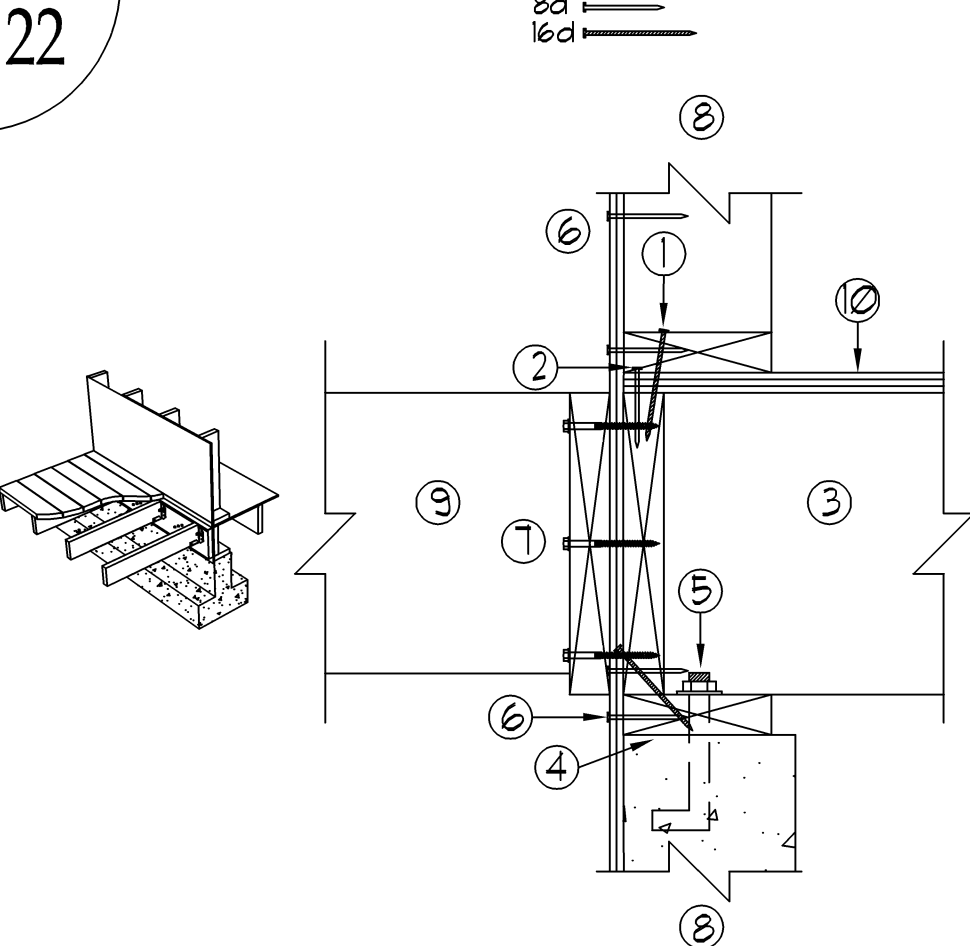


26

NOT USED

26  
D2

22



8d  
16d

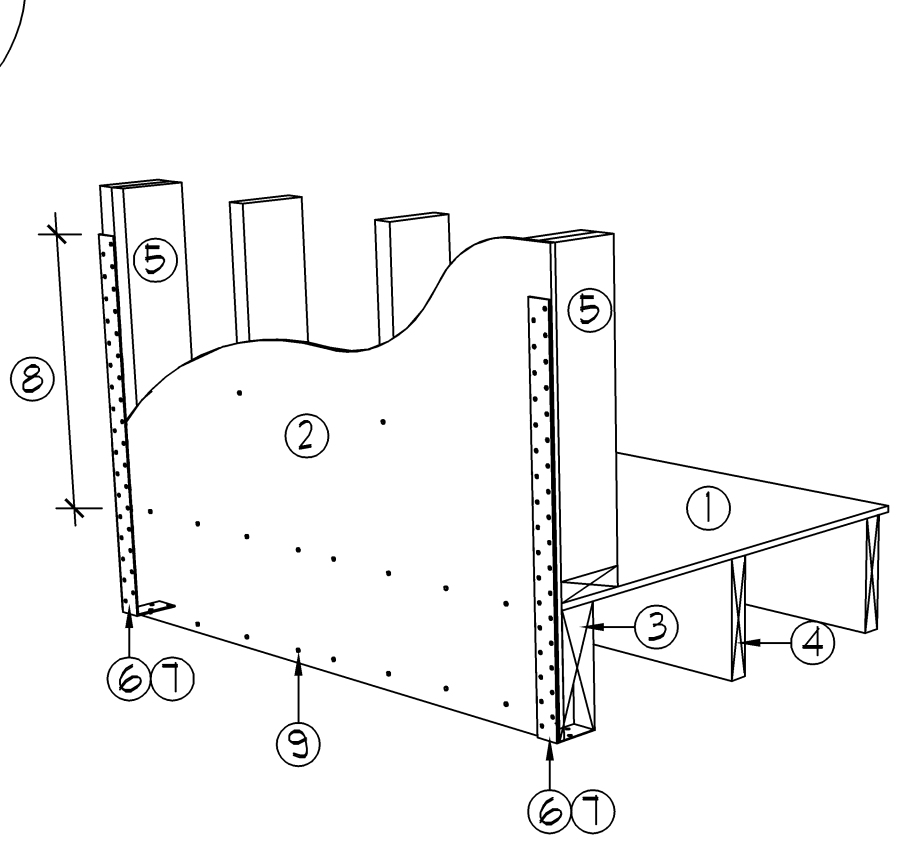
- 1 2x SOLE PLATE TO RIM W/16d @ 6" O.C. UNO.
- 2 3/4 T&G TO RIM: 8d @ 6" O.C.
- 3 2x OR MFR FLOOR JOIST
- 4 RIM TO MUD SILL W/16d @ 4" O.C. UNO.
- 5 ANCHOR BOLT PER PLAN
- 6 SHEATHING TO STUD SEE SHEAR WALL SCHEDULE
- 7 (3) SDUS22400DB SCREWS STAGGERED @ 16" O.C.
- 8 CONCRETE FOUNDATION
- 9 DECK JOISTS PER PLAN
- 10 3/4" T & G FLOORING

\* SEE SHEARWALL TABLE FOR ADDITIONAL NAILING

22  
D2

DECK LEDGER CONNECTION

18

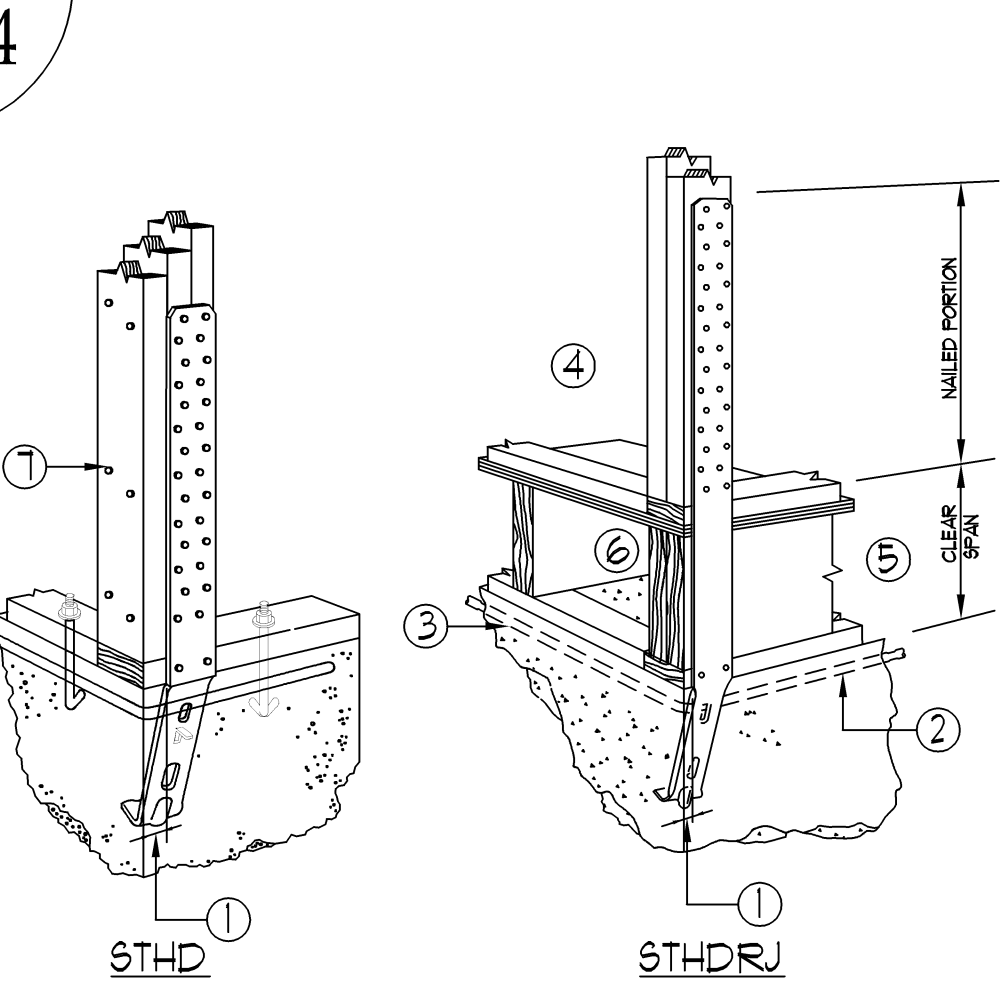


- 1 3/4" T&G PLYWOOD SUBFLOOR
- 2 SHEAR WALL NAILING PER PLAN
- 3 FLUSH DOUBLE 2x JOIST OR MFR DOUBLE JOIST WITH WEB STIFFENERS OR BEAM PER PLAN
- 4 FLOOR JOIST PER PLAN
- 5 STUD/POST PER SHEAR WALL SCHEDULE
- 6 MSTC48B3 (WHEN SPECIFIED ON PLAN)
- 7 (2)CS14 OPTION (36" MIN LENGTH, WRAP AROUND BEAM)
- 8 18" MIN TO STUD WALL (CS14 OPTION)
- 9 NAIL SHEATHING TO BEAM. DO NOT BREAK SHEATHING

18  
D2

SHEAR WALL AT BEAM

14



- 1 1/2" MIN FROM CORNER
- 2 (1) #4 REBAR. MAY BE FOUNDATION REBAR OR POST TENSION
- 3 24" MIN REBAR LENGTH
- 4 NOTE: HOLDOWNS AT CORNER ARE SPACED 1-1/2" FROM EDGE. SPACE 1/2" WHEN 2x IS FLAT.
- 5 CLEAR SPAN IT" MAXIMUM. FOR TYPE RJ HOLDOWN 0" FOR NON RJ HOLDOWNS
- 6 COMPRESSION BLOCKING
- 7 (2) 16d NAILS @ 12" oc

BUILDER TO DETERMINE IF STHD OR STHDRJ IS APPROPRIATE.

14  
D2

SIMPSON STHD

27

NOT USED

27  
D2

23

NOT USED

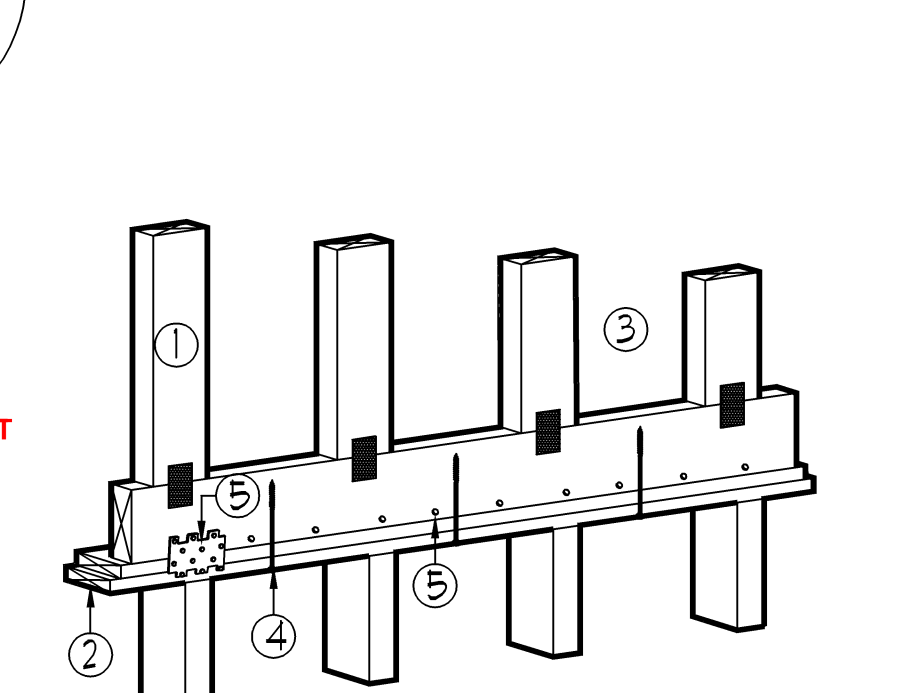
BASIC PERMIT PACKAGE  
REVIEWED FOR CODE COMPLIANCE  
WITH IRC 2015  
KITSAP COUNTY BUILDING DEPARTMENT

08/02/2018 11:56:07 AM  
#1 18-02763 ]

Reviewed for code compliance  
with IRC 2015  
Kitsap County Building Department  
jbowling@co.kitsap.wa.us  
07/21/2020

23  
D2

19



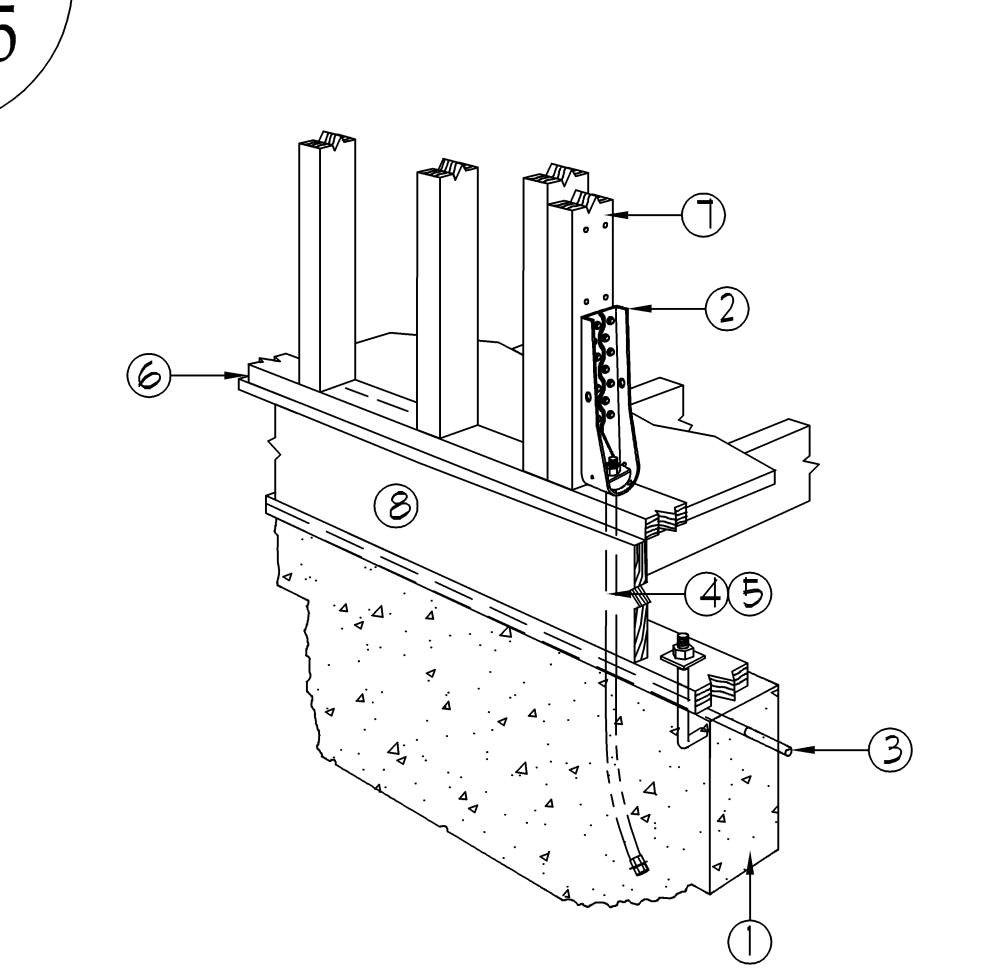
- 1 MFR GABLE END TRUSS
- 2 DOUBLE TOP PLATE (UNO. ON PLAN)
- 3 CONTINUOUS VERTICAL SHEATHING OVER GABLE FRAMING
- 4 6" TIMBERLOK OR 6" SDWC AT 16" O.C.
- 5 TOP PLATE TO BOTTOM CHORD CONNECTION PER SHEARWALL SCHEDULE

NOTES:  
-INSTALL SCREWS UPWARD THROUGH TOP PLATES INTO CENTER OF BOTTOM TRUSS CHORD.  
-BRACE TRUSSES PER BCSI. CONSULT TRUSS MANUFACTURER FOR BRACING REQUIREMENTS.

19  
D2

GABLE END TRUSS BRACING

15



- 1 CONCRETE STEM WALL PER PLAN
- 2 SIMPSON HDU2 OR HDU5 HOLDOWN PER PLAN
- 3 #4 REBAR FOR TOP REINFORCEMENT
- 4 SIMPSON 98B/8x24 ANCHOR
- 5 SIMPSON 98TB16 OPTION FOR HDU2 APPLICATIONS
- 6 MUDSILL W/ ANCHOR BOLTS PER PLAN
- 7 DOUBLE STUD W/ (2) 8d @ 8"oc
- 8 NO RIM REQUIRED WHEN SLAB ON GRADE

15  
D2

HDU2 & HDU5 CONNECTION

28

NOT USED

28  
D2

24

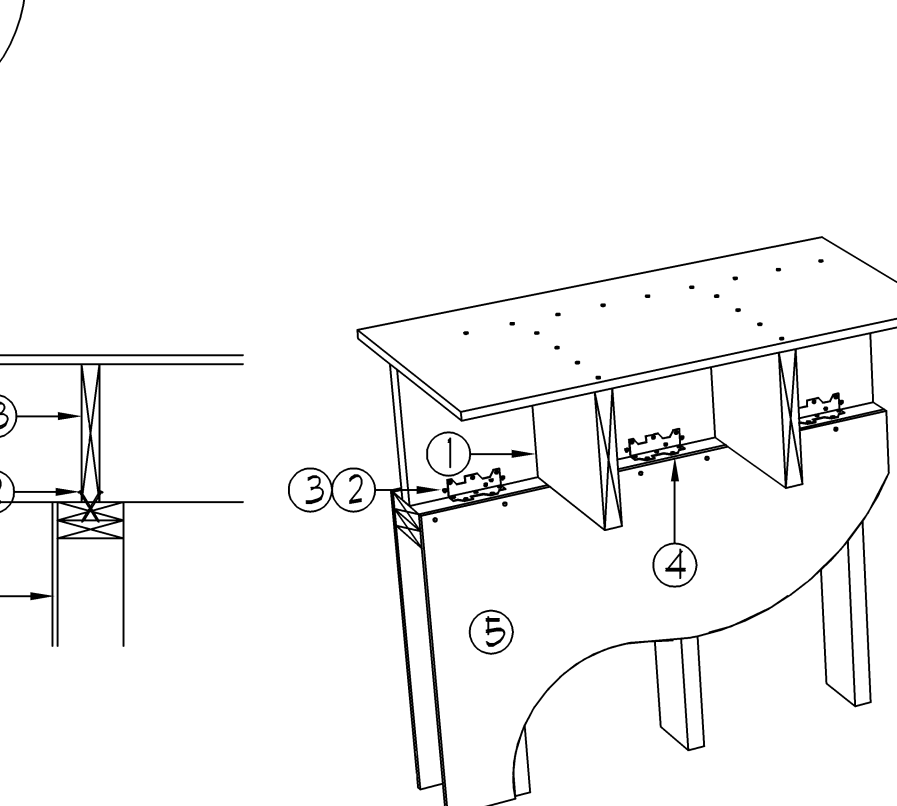
Subject To Field Inspection

NOT USED

CHANGES  
MUST Be Approved Prior  
To Performing Work

24  
D2

20



SECTION VIEW

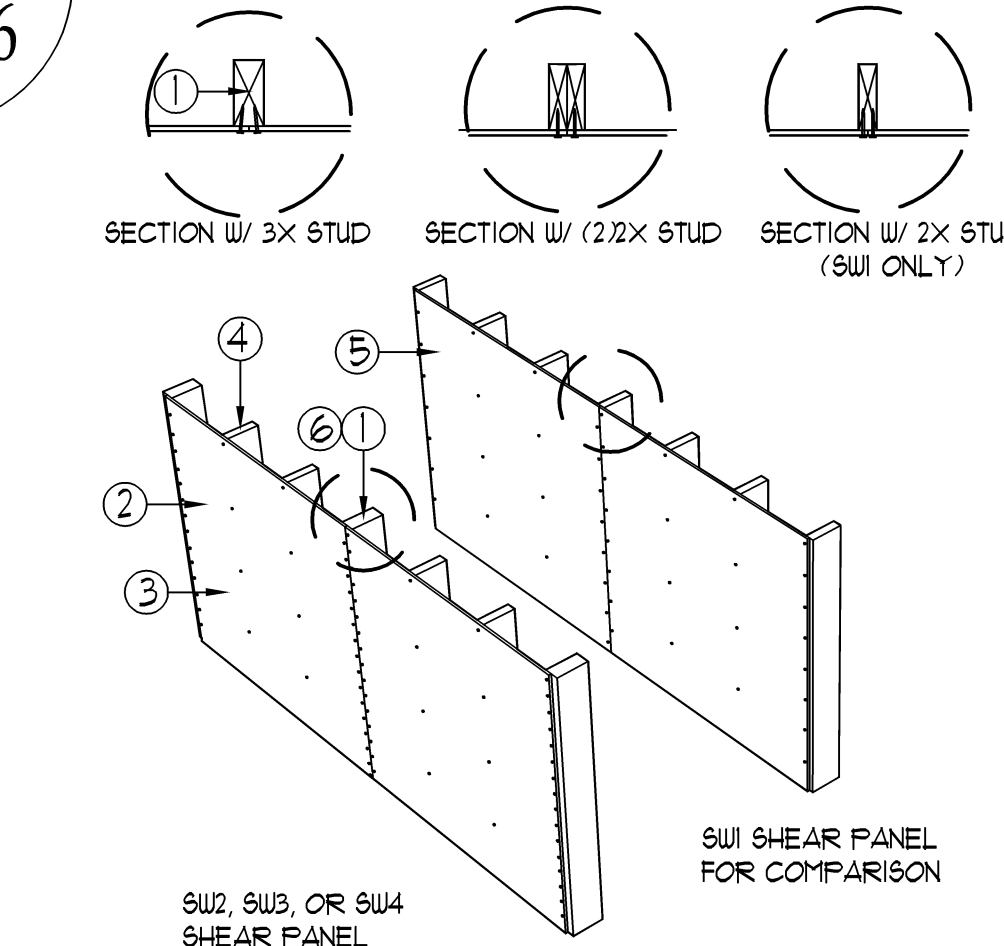
OVERALL PERSPECTIVE

- 1 2x FLOOR JOIST OR MFR JOIST. SPACING & SIZE PER PLAN
- 2 (3) 8d TOE NAILS EACH SIDE EVERY 16" ON CENTER
- 3 2x BLOCKING CENTERED ON DOUBLE TOP PLATE. FULL DEPTH
- 4 SIMPSON A35 REQUIRED FOR SW2 & SW3 SHEAR WALLS. (2) SIMPSON A35 AT SW4 & SW6 SHEAR WALLS (STAGGER AT EACH SIDE OF BLOCKING)
- 5 INTERIOR SHEAR WALL NAILING & SHEATHING PER PLAN.

20  
D2

INTERIOR SHEARWALL TO PERPENDICULAR JOISTS ABOVE

16



SECTION W/ 3x STUD

SECTION W/ (2)2x STUD

SECTION W/ 2x STUD (SUI ONLY)

SW2, SW3, OR SW4 SHEAR PANEL

SUI SHEAR PANEL FOR COMPARISON

- 1 3x MEMBER AT ADJOINING PANEL EDGES FOR SW3 AND SW4 SHEAR WALL
- 2 SW2 SHEAR WALL. 8d NAIL: 4" EDGE - 12" FIELD  
SW3 SHEAR WALL. 8d NAIL: 3" EDGE - 12" FIELD  
SW4 SHEAR WALL. 8d NAIL: 2" EDGE - 12" FIELD
- 3 7/16" OSB SHEATHING
- 4 2x MEMBER FOR SHEAR WALLS MID PANEL (FIELD)
- 5 SHEAR WALL PER PLAN
- 6 (2) 2x MAY BE USED IN PLACE OF 3x PER SHEAR WALL NOTES

16  
D2

SW2, SW3, SW4 SHEAR WALL NAILING

29

NOT USED

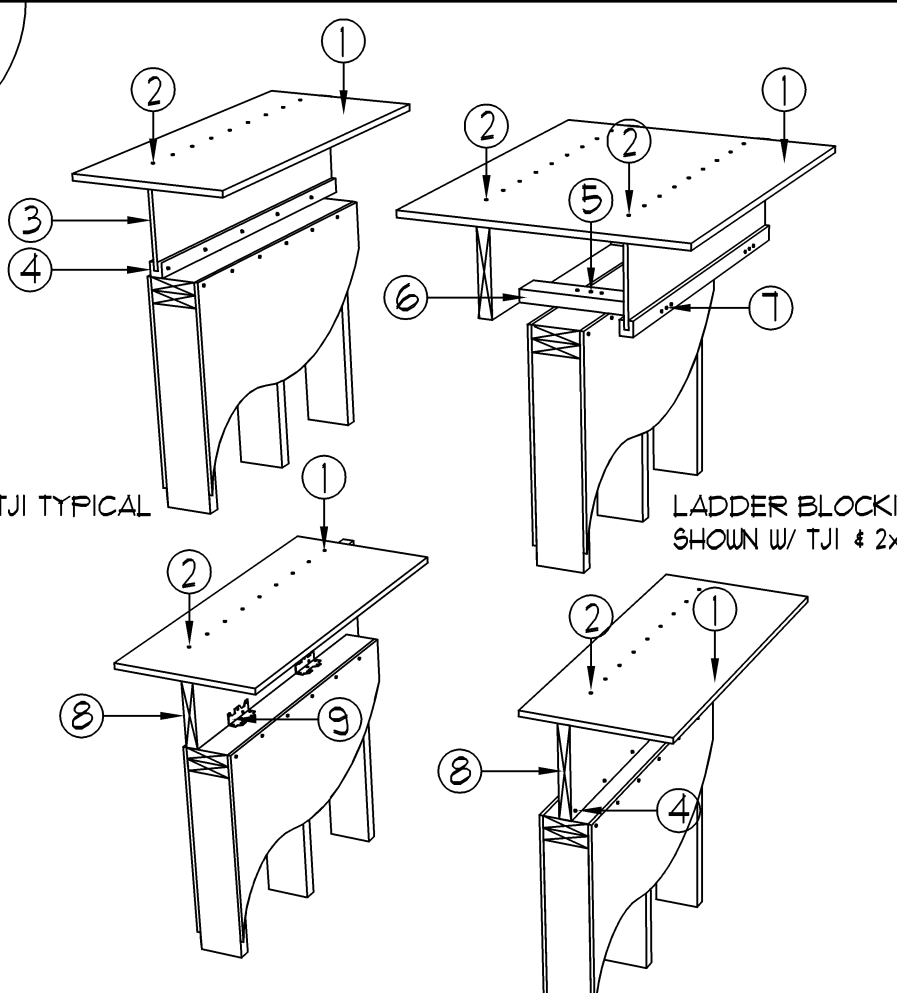
29  
D2

25

NOT USED

25  
D2

21



TJI TYPICAL

LADDER BLOCKING SHOWN W/ TJI & 2x

DIMENSIONAL W/ SIMPSON A35

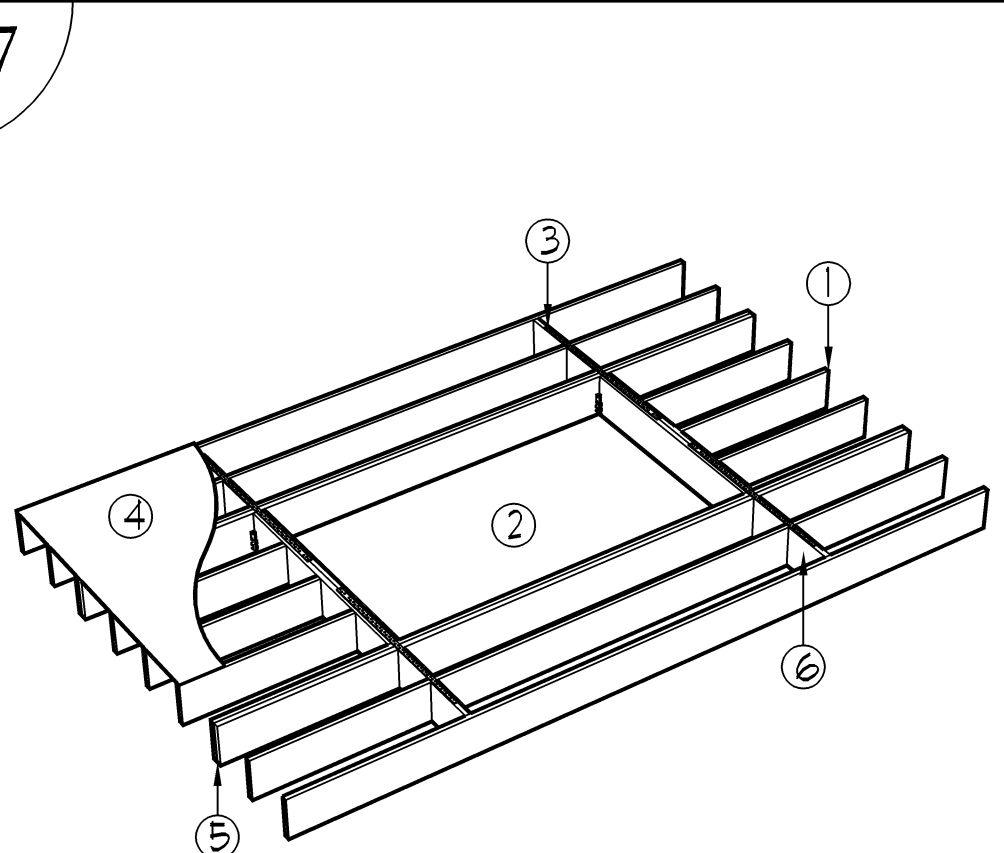
DIMENSIONAL TYPICAL

- 1 3/4" T&G FLOOR
- 2 8d @ 4" O.C.
- 3 TJI PARALLEL OR TJI BLOCKING
- 4 8d @ 4" O.C. STAGGER SIDES. NAIL THROUGH TOP OF FLANGE. DO NOT TOE-NAIL
- 5 (3) 16d NAILS
- 6 2x4 FLAT @ 16" O.C.
- 7 (3) 8d NAILS EACH END
- 8 FLOOR JOISTS OR SOLID BLOCKING
- 9 A35 @ 16" O.C. OPTION

21  
D2

INTERIOR SHEAR WALL TO JOIST ABOVE CONNECTION

17



- 1 FLOOR JOIST PER PLAN
- 2 DIAPHRAGM OPENING PER PLAN
- 3 TIE & BLOCKING EQUAL TO WIDTH OF OPENING. CS14 OR SIMILAR
- 4 SHEATHING PER PLAN
- 5 BEAM/DBL JOIST PER PLAN AROUND OPENING (TYPICAL 4 PLACES) (UNO. ON PLAN)
- 6 BLOCKING FOR STRAP

17  
D2

LARGE DIAPHRAGM OPENING

REVISIONS

NO.	DATE	REVISIONS

DO NOT SCALE

Any variations from conditions and dimensions shown on the drawings are the responsibility of the designer and engineer for the drawings. The contractor shall be responsible for the costs of any necessary remedial work.

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j.hodge@hodgeengineering.com

**Hodge** inc.  
engineering

**FERGUSON & COLE, INC**  
**2800**  
**SITE SPECIFIC LETTER**  
**REQUIRED FOR EACH USE**

BUILDING CODE 2015 IBC

WIND EXP. B 110 MPH

SEISMIC DESIGN CATEGORY D

ROOF SNOW LOAD 25 PSF

ENG. P.E. JOHN H.

DATE 05.31.18

REVIEW ZACK C.

ANALYST ANNA N.

SHEET

**D2**

3 OF 4 SHEETS

PROJECT NUMBER 180558

JOHN E. HODGE  
REGISTERED PROFESSIONAL ENGINEER  
NO. 37888  
STATE OF NORTH DAKOTA  
06/04/2018

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HODGE ENGINEERING, INC.



1. GENERAL STRUCTURAL NOTES

HODGE ENGINEERING HAS BEEN CONTRACTED TO PROVIDE LATERAL ENGINEER ONLY. HODGE ENGINEERING IS NOT THE ENGINEER OF RECORD. QUESTIONS FOR THE ENGINEER OF RECORD SHOULD BE DIRECTED TO THE DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. CONFLICTS BETWEEN THE LATERAL ENGINEERING, DETAILS, AND NOTES WITH THE DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE'S DRAWINGS, DETAILS, AND NOTES SHALL BE IDENTIFIED TO HODGE ENGINEERING AS SOON AS IDENTIFIED.

THIS LATERAL ENGINEERING HAS BEEN CALCULATED TO RESIST CODE SPECIFIED WIND & SEISMIC FORCES AFTER THE COMPLETION OF ALL STRUCTURAL ELEMENTS. IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO ENSURE STRUCTURAL STABILITY DURING CONSTRUCTION INCLUDING JOB SITE SAFETY; ERECTION METHODS; TEMPORARY SHORING, FORM WORK, AND BRACING; USE OF EQUIPMENT AND CONSTRUCTION PROCEDURES. ANY DISCREPANCIES FOUND AMONG THE DRAWINGS, ENGINEERING, AND/OR THE SITE CONDITIONS MUST BE REPORTED TO THE ARCHITECT AND ENGINEER, WHO SHALL CORRECT SUCH DISCREPANCY IN WRITING. ANY WORK DONE BY THE GENERAL CONTRACTOR AFTER DISCOVERY OF SUCH DISCREPANCY SHALL BE DONE AT THE GENERAL CONTRACTOR'S RISK. THE GENERAL CONTRACTOR SHALL VERIFY AND COORDINATE ALL DIMENSIONS PRIOR TO CONSTRUCTION. CONSTRUCTION OBSERVATION BY THE STRUCTURAL ENGINEER IS NOT PROVIDED. THIS ENGINEERING IS NOT INTENDED IN ANY WAY TO REVIEW OR APPROVE THE CONTRACTOR'S CONSTRUCTION PROCEDURES.

STANDARDS

ALL METHODS, MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE 2015 INTERNATIONAL RESIDENTIAL CODE (IRC) AS AMENDED AND ADOPTED BY THE LOCAL BUILDING OFFICIAL OR APPLICABLE JURISDICTION. THIS ENGINEERING IS TO THE 2015 INTERNATIONAL BUILDING CODE (IBC)

2. DESIGN CRITERIA

VERTICAL LOADS

IBC TABLE 1607.1	
MINIMUM UNIFORMLY DISTRIBUTED LIVE LOADS	
USE	LIVE LOAD
ATTICS WITH STORAGE (UNINHABITABLE)	20 PSF
ATTICS WITHOUT STORAGE (UNINHABITABLE)	10 PSF
DECKS	60 PSF
EXTERIOR BALCONIES	60 PSF
FIRE ESCAPES	40 PSF
GUARD RAILS AND HAND RAILS	200 PSF
GUARD RAIL INFILL COMPONENTS	50 PSF
PASSENGER VEHICLE GARAGES	40 PSF
ROOMS OTHER THAN SLEEPING ROOMS	40 PSF
SLEEPING ROOMS	30 PSF
STAIRS	40 PSF

LATERAL FORCES

WIND:

ASCE 7-10 DIRECTIONAL PROCEDURE FOR BUILDINGS OF ALL HEIGHTS

SEISMIC:

SEISMIC IMPORTANCE FACTOR PER ASCE 7-10, IE = 1.00  
BUILDING OCCUPANCY RISK CATEGORY PER ASCE 7-10 TABLE 1.5-1 = II  
SOIL SITE CLASS PER ASCE 7-10 TABLE 20.3-1 = PER PLAN  
SEISMIC DESIGN CATEGORY IBC TABLE 1613.3.5(1) & 1613.3.5(2) = PER PLAN  
ANALYSIS PROCEDURE USED = EQUIVALENT LATERAL FORCE ANALYSIS  
RESPONSE MODIFICATION FACTOR R = 6.5

3. SOIL

HODGE ENGINEERING IS NOT THE ENGINEER OF RECORD. ALL SOIL REQUIREMENTS ARE TO BE DETERMINED BY THE DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE.

4. CONCRETE

HODGE ENGINEERING HAS BEEN CONTRACTED TO PROVIDE LATERAL ENGINEERING ONLY. THE CONCRETE FOUNDATION AND RETAINING WALLS IF REQUIRED ARE TO BE DESIGNED BY THE DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE.

CONCRETE: SHALL BE MADE WITH PORTLAND CEMENT ASTM C-150 TYPE II OR TYPE I AND SHALL BE READY MIXED PER ASTM C-94.

MIX DESIGNS: THE CONTRACTOR SHALL DESIGN CONCRETE MIXES THAT MEET THE REQUIREMENTS OF THE DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. THE MIX DESIGNS SHALL FACILITATE ANTICIPATED PLACEMENT METHODS, WEATHER, TEMPERATURE, REBAR CONGESTION, AND ALL OTHER FACTORS REQUIRED TO PROVIDE A STRUCTURALLY SOUND AND ACCEPTABLE FINISHED PRODUCT. WATER REDUCING ADMIXTURES MAY BE USED TO MEET THESE REQUIREMENTS. MAXIMUM SLUMP SHALL BE 5".

ADMIXTURES: ADMIXTURES SHALL BE BY MASTER BUILDERS, W.R. GRACE, OR PRE-APPROVED EQUAL. ALL MANUFACTURER'S RECOMMENDATIONS SHALL BE FOLLOWED.

WATER: SHALL BE CLEAN AND POTABLE

CONCRETE STRENGTH REQUIREMENTS ARE TO BE PROVIDED BY THE DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. THIS LATERAL ENGINEERING REQUIRES A MINIMUM OF 2500 PSI @ 28 DAYS.

REINFORCING STEEL: SHALL CONFORM TO ASTM A-706, GRADE 60. PLACE PER ACI 315 AND ACI 318. VERTICAL AND HORIZONTAL REINFORCEMENT SHALL BE THE LONGEST LENGTHS PRACTICAL. WHERE SPLICES ARE NECESSARY THE LENGTH OF LAP SPLICE SHALL BE A MINIMUM OF 30 INCHES FOR #4, 38 INCHES FOR #5, AND 45 INCHES FOR #6. THE MAXIMUM GAP BETWEEN NONCONTACT PARALLEL BARS AT A LAP SPLICE SHALL NOT EXCEED 5 INCHES. PROVIDE CORNER BARS AT ALL HORIZONTAL BARS IN FOOTING AND WALLS (1907).

CRACKS: UNREINFORCED CONCRETE WILL CRACK.

CONCRETE MINIMUM COVER OVER REINFORCEMENT:  
CONCRETE CAST AGAINST EARTH = 3" EXPOSED TO WEATHER OR EARTH = 2"  
WALLS AND SLABS NOT EXPOSED TO WEATHER = 3/4"

BOLTS

MACHINE BOLTS (M.B.): ASTM A-307, GRADE A  
ANCHOR BOLTS (A.B.): ASTM F1554, GRADE 36, CLASS 2A

5. CARPENTRY

NAILS: CONNECTION DESIGNS ARE BASED ON "COMMON WIRE" NAILS WITH THE FOLLOWING PROPERTIES:

PENNYWEIGHT	DIAMETER (IN.)	LENGTH (IN.)
8d	0.131	2-1/2
10d	0.148	3
16d	0.162	3-1/2
20d	0.192	4

WOOD SHEATHING (STRUCTURAL): SHEATHING SHALL BE PLYWOOD (CDX) OR ORIENTED STRAND BOARD (OSB). PLYWOOD SHEATHING SHALL BE 5-PLY MINIMUM WHERE INDICATED AS 3/4" OR THICKER. WOOD SHEATHING SHALL BE "RATED SHEATHING" CONFORMING TO PS1-95 AND/OR PS2-92. ALL PANELS SHALL BEAR THE STAMP OF AN APPROVED GRADING AGENCY. SPAN RATING SHALL BE PROVIDED AS FOLLOWS: ROOF (32/16); WALLS (32/16); FLOORS (20" O.C.) UNLESS NOTED OTHERWISE. ALL WOOD SHEATHED WALLS SHALL BE BLOCKED AT ALL PANEL EDGES UNLESS NOTED OTHERWISE.

GLUE-LAMINATED MEMBERS: PER ANSI/AITC A190.1. MEMBERS SHALL BE COMBINATION 24F-V4 DOUGLAS FIR FOR SIMPLE SPANS AND 24F-V8 DOUGLAS FIR FOR CANTILEVERED SPANS AND TRUSS CHORDS (FB=2400 PSI, FV=240 PSI, E=1.8X10^6 PSI) AND DOUGLAS FIR COMBINATION 2 FOR COLUMNS AND TRUSS WEB MEMBERS, ALL WITH EXTERIOR GLUE. ALL MEMBERS TO HAVE AITC OR APA-EWS STAMP.

FRAMING LUMBER:  
EACH PIECE SHALL BEAR THE GRADE TRADEMARK OF THE WEST COAST LUMBER INSPECTION BUREAU (WCLIB), WESTERN WOOD PRODUCTS ASSOCIATION (WWPA), OR OTHER AGENCY ACCREDITED BE THE AMERICAN LUMBER STANDARD COMMITTEE (ALSC) TO GRADE UNDER ALSC CERTIFIED GRADING RULES.

		APPLICATION				SPECIFIED MATERIAL		PRESERVATIVE TREATMENT (1)		CONNECTORS & FASTENERS (2)(3)	
		FOUNDATION SILL PLATES, TOP PLATES & LEDGERS ON CONCRETE OR MASONRY WALLS (4)		2X, 4X, 6X OR GLULAM (FIR)		SBX		GALV (G60)			
EXPOSURE	DRY					ACQ, CBA, CA		GALV (G185)			
				2X, & 4X (FIR)		ACQ, CBA, CA		GALV (G185)			
	WET	FRAMING, DECKING, POSTS & LEDGERS		2X, & 4X (CEDAR)		NONE		GALV (G90)			
				6X OR GLULAM (FIR)		ACQ, CBA, CA		GALV (G185)			
				6X OR GLULAM (CEDAR)		NONE		GALV (G90)			
		BEAMS AND COLUMNS									

PRESERVATIVE TREATED WOOD FASTENER REQUIREMENTS:

ALL METAL CONNECTORS COMING IN CONTACT WITH PRESSURE TREATED WOOD SHALL BE SIMPSON Z-MAX, TRIPLE ZINC COATED, OR HOT DIPPED GALVANIZED. CCA: CHROMATED COPPER ARSENATE NOT PERMITTED.  
SBX: DOT SODIUM BORATE  
ACQ: ALKALINE COPPER QUAT  
CBA AND CA: COPPER AZOLE

G60, G90, AND G185 PER ASTM A23 FOR CONNECTORS, AND ASTM A153 FOR FASTENERS. MECHANICALLY GALVANIZED FASTENERS PER ASTM B695, CLASS 55 OR GREATER.

POSITIVE CONNECTIONS SHALL BE PROVIDED AT POST-BEAM CONNECTIONS TO ENSURE AGAINST UPLIFT AND LATERAL DISPLACEMENT.

BASIC PERMIT PACKAGE  
REVIEWED FOR CODE COMPLIANCE  
WITH IRC 2015  
KITSAP COUNTY BUILDING DEPARTMENT

08/02/2018 11:56:07 AM  
#[ 18-02763 ]

Reviewed for code compliance  
with IRC 2015  
Kitsap County Building Department  
jbowling@co.kitsap.wa.us  
07/21/2020

Subject To Field Inspection

CHANGES  
MUST Be Approved Prior  
To Performing Work

NO.	DATE	REVISIONS

drawings shall carry the date of revision. If the drawing is not to be issued, DO NOT SCALE. Any variations from conditions and dimensions shown on the drawings shall be approved by the designer and engineer for the contractor. The contractor shall be responsible for the costs of any necessary remedial work.

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**Hodge** inc.  
engineering

FERGUSON & COLE, INC

2800

SITE SPECIFIC LETTER  
REQUIRED FOR EACH USE

BUILDING CODE	2015 IBC
WIND EXP.	B 110 MPH
SEISMIC DESIGN CATEGORY	D
ROOF SNOW LOAD	25 PSF

ENG. P.E. JOHN H.

DATE 05.31.18

REVIEW ZACK C.

ANALYST ANNA N.

SHEET

N1

4 OF 4 SHEETS

PROJECT NUMBER 180558

Professional Engineer  
John Hodge, P.E.  
06/04/2018  
180558  
HODGE ENGINEERING, INC.  
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