# CLIENT

ENVISION NORTHWEST, LLC AUSTIN ROUPE

10829 NE 68TH STREET, SUITE B

KIRKLAND, WA 98033

## ADM ARCHITECTUR AARON MURPHY, AF 5819 NE MINDER RO POULSBO, WA 98370

Reviewed for code complia. with IRC 2015 Kitsap County Building Departme. lasmith@co.kitsap.wa.us

revised

## BASIC PERMIT PACKAGE REVIEWED FOR CODE COMPLIANCE WITH IRC 2015 KITSAP COUNTY BUILDING DEPARTMENT

## GOVERNING DESIGN CODE

1. 2015 INTERNATIONAL BUILDING CODE WITH WASHINGTON STATE AMENDMENTS.

## DESIGN CRITERIA

- 1. DESIGN LIVE AND SNOW LOADS FOR NEW CONSTRUCTION, UNLESS NOTED OTHERWISE:
- 2. LIVE LOADS: ROOF 20 PSF MINIMUM ROOF LIVE LOAD (REDUCIBLE)
- RESIDENTIAL FLOORS 40 PSF
- DECKS 60 PSF
- 3. SNOW LOADS:
- ROOF SNOW LOAD, P<sub>S</sub>: 25 PSF GROUND SNOW LOAD, P<sub>G</sub>: 30 PSF
- FLAT ROOF SNOW LOAD, P<sub>F</sub>: 21 PSF
- SNOW EXPOSURE "B"
- EXPOSURE FACTOR, C<sub>E</sub>: 1.0
- IMPORTANCE FACTOR, I: 1.0
- THERMAL FACTOR, C<sub>T</sub>: 1.0
- 5. SUPERIMPOSED DEAD LOADS
- ROOF: 15 PSF

## FLOOR: 15 PSF 6. WIND DESIGN DATA:

- ALTERNATE ALL-HEIGHTS METHOD 2015 UBC 1609.6
- ULTIMATE WIND SPEED 110 MPH
- EXPOSURE "B"
- WIND DIRECTIONALITY FACTOR, Kd: 0.85
- TOPOGRAPHIC FACTOR, K<sub>71</sub>: 1.0 • VELOCITY PRESSURE FACTOR, Kz: 0.65
- 7. EARTHQUAKE DESIGN DATA:
- SEISMIC DESIGN CATEGORY: D
- SEISMIC RISK CATEGORY II
- IMPORTANCE FACTOR, I: 1.0
- BASIC SEISMIC-FORCE RESISTING SYSTEMS(S)
- BEARING WALL SYSTEM LIGHT FRAME WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE. SOIL SITE CLASS D
- MAPPED RESPONSE ACCELERATIONS; S<sub>S</sub>: 1.39, S<sub>D</sub>: 0.55. SPECTRAL RESPONSE COEFFICIENTS; S<sub>DS</sub>: 0.93, S<sub>D1</sub>: 0.55
- SEISMIC RESPONSE COEFFICIENT(S), C<sub>S</sub>: 0.14
- RESPONSE MODIFICATION FACTOR(S), R: 6.5
- **GENERAL NOTES**
- 1. THESE STRUCTURAL NOTES ARE A SUPPLEMENT TO THE SPECIFICATIONS. 2. SPECIFICATIONS AND CODES REFERENCED IN THESE NOTES ARE THE VERSIONS MOST RECENTLY ADOPTED BY THE PERMITTING
- AUTHORITY.
- 3. VERIFY DIMENSIONS AND CONDITIONS WITH THE ARCHITECTURAL DRAWINGS. FIELD VERIFY DIMENSIONS AND ELEVATIONS RELATIVE TO THE EXISTING STRUCTURE PRIOR TO FABRICATION OF MATERIALS.
- 4. FOR FEATURES OF CONSTRUCTION NOT FULLY SHOWN, PROVIDE THE SAME TYPE AND CHARACTER AS SHOWN FOR SIMILAR CONDITIONS, SUBJECT TO REVIEW BY THE ARCHITECT AND ENGINEER OF RECORD.
- 5. APPLY, PLACE, ERECT OR INSTALL ALL PRODUCTS AND MATERIALS IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS.
- 6. ADEQUATELY BRACE STRUCTURE AND ALL STRUCTURAL COMPONENTS AGAINST WIND, LATERAL EARTH AND SEISMIC FORCES UNTIL THE
- PERMANENT LATERAL-FORCE RESISTING SYSTEMS HAVE BEEN INSTALLED. 7. PROVIDE BLOCKING BETWEEN STUDS (OR OTHER MEANS OF BRACING) AT WOOD BEARING WALLS TO PREVENT STUD BUCKLING PRIOR TO INSTALLATION OF GYPSUM WALLBOARD.

## SITE PREPARATION

- 1. REMOVE VEGETATION, RUBBISH AND EXISTING FILL WITHIN BUILDING FOOTPRINT AND 5-0" (MINIMUM) BEYOND THE FOOTPRINT. STRIP TOP SOIL 6", MINIMUM.
- 2. PRE-ROLL AREA WITHIN BUILDING FOOTPRINT AND 5'-0" (MINIMUM) BEYOND THE FOOTPRINT WITH A HEAVY VIBRATORY ROLLER OR
- LOADED DUMP TRUCK. MAKE THREE PASSES (MINIMUM) OVER THE ENTIRE AREA. 3. REMOVE AREAS OF SOIL, AS REQUIRED, THAT EXHIBIT EXCESSIVE WEAVING OR DEFLECTION UNDER THE WEIGHT OF THE ROLLER OR
- DUMP TRUCK. 4. BACK-FILL EXCAVATED AREAS WITH STRUCTURAL FILL AS DESCRIBED BELOW.
- 5. CONSULT THE FOUNDATION INVESTIGATION REPORT, AVAILABLE AT ARCHITECT'S OFFICE, FOR ADDITIONAL INFORMATION.

## STRUCTURAL FILL OR BACK-FILL

- 1. STRUCTURAL FILL MATERIAL:
- SAND AND GRAVEL MIXTURE OR CRUSHED ROCK. • WELL GRADED FROM COARSE-TO-FINE WITH LESS THAN 10% BY WEIGHT OF THE MINUS 3/4" FRACTION PASSING THE NO. 200 SIEVE.
- FREE OF ORGANICS, RUBBISH, CLAY BALLS AND ROCKS LARGER THAN 4".
- PLACE STRUCTURAL FILL IN LOOSE LIFTS, MAXIMUM OF 8" IN THICKNESS.
- COMPACT STRUCTURAL FILL TO A MINIMUM DENSITY OF 95% OF MAXIMUM DRY DENSITY, AS DETERMINED BY ASTM D 1557.
- VERIFY ADEQUACY OF STRUCTURAL FILL COMPACTION WITH RANDOM FIELD DENSITY TESTS.

## **FOUNDATIONS**

Established Basic Permit #

19-03671R2

- 1. FOUNDATION SIZES BASED ON AN ALLOWABLE SOIL BEARING PRESSURE OF 2,500 PSF DEAD AND LIVE LOADS.
- 2. FOUNDATION ELEVATIONS WHERE SHOWN ARE TO TOP OF FOOTINGS.
- 3. PLACE FOOTINGS ON FIRM, UNDISTURBED ORIGINAL SOIL, OR ON STRUCTURAL FILL. SEE "STRUCTURAL FILL OR BACK-FILL" NOTES FOR STRUCTURAL FILL INFORMATION.
- 4. LOCATE BOTTOM OF FOOTINGS AT A MINIMUM OF 1'-0" BELOW FINAL GRADE OR 1'-0" BELOW EXISTING GRADE, WHICHEVER IS LOWER.
- 5. PRIOR TO PLACEMENT OF CONCRETE, REMOVE ALL DISTURBED SOIL FROM FOOTING EXCAVATION TO NEAT LINES. 6. STEP BOTTOM OF FOOTINGS FROM ELEVATION TO ELEVATION AT A RATIO OF 1 VERTICAL TO 2 HORIZONTAL, WITH A MAXIMUM VERTICAL STEP OF 2'-0".

- LAB. ERECT I PLYWOOD SHEA 1. PLYWOOD MA PRODUCT ST ASSOCIATION CONFORMS BEARS THE 2. PROVIDE PRE AWPB
- •• ROC FLO •• •• WAI
- 3. PLYWOOD LA OUT PLYWOC PROVIDE PAN
- EDGES. FAST
- 4. PROTECT FL

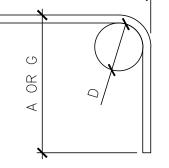
ARCHITECT		DRAWING INDEX	INC
ARCHITECTURE, LLC	S1.0 S <sup>-</sup>	TRUCTURAL NOTES & PROJECT INFORMATION	
N MURPHY, ARCHITECT	S1.1 S <sup>-</sup>	TRUCTURAL SCHEDULES	THWEST GROUP, INEERING CO
NE MINDER ROAD, SUITE "G"		HEARWALL & HOLDOWN PLANS & SCHEDULES	
SBO, WA 98370		OUNDATION/FIRST FLOOR PLAN LTERNATE FOUNDATION/FIRST FLOOR PLAN	
	$\mathbf{b}$	ECOND FLOOR FRAMING PLAN	
Validity of permit. The issuance or granting of a permit shall not be construed to be a permit for, or an approval of, any violation of any of the provisions of the International Codes or any other ordinance	$\geq$	LTERNATE SECOND FLOOR FRAMING PLAN	
of the provisions of the International Codes or any other ordinance of Kitsap County. Permits presuming to give authority to violate or cancel the provisions of the International Codes and ordinances of Kitsap County shall not be valid. IBC & IRC 105	S2.3 TH	HIRD FLOOR ROOF FRAMING PLAN	NC ALE
		LTERNATE THIRD FLOOR FRAMING PLAN	
IENT     Must Comply With All Washington     CHANGES MUST Be Approved Prior To Performing Work		PPER ROOF FRAMING PLAN	
State Codes Subject To Field Inspection		ECTIONS & DETAILS ECTIONS & DETAILS	LIFI UC FESS, MALBON 1, WA 920 803
		ECTIONS & DETAILS	NE N STON
STRUCTURAL NOTES		ECTIONS & DETAILS YPICAL SECTIONS & DETAILS	
CONCRETE REINFORCING STEEL			
<ol> <li>REINFORCING STEEL (TYPICAL, UNLESS NOTED OTHERWISE): ASTM A 615, GRADE 60.</li> <li>DETAIL, FABRICATE AND PLACE REINFORCING ACCORDING TO ACI 315, "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT".</li> </ol>		ENGINEERED WOOD PRODUCTS 1. CONFORM WITH ALL APPLICABLE PROVISIONS OF THE IBC.	
<ul> <li>TYPICAL REINFORCING (MINIMUM, UNLESS NOTED OTHERWISE ON DRAWINGS):</li> <li>CORNERS AND INTERSECTIONS OF WALLS AND FOUNDATIONS: CORNER BARS EQUAL IN SIZE AND NUMBER TO HORIZONTAL</li> </ul>		<ol> <li>WOOD PRODUCT MANUFACTURER: TRUS JOIST, A WEYERHAEUSER BUSINESS, OR APPROVED.</li> <li>TJI SERIES JOISTS:</li> </ol>	
REINFORCING. LEG LENGTH: 48 BAR DIAMETERS (2'-0" MINIMUM). 4. DO NOT FIELD BEND, DISPLACE, WELD, HEAT OR CUT REINFORCING UNLESS INDICATED ON THE DRAWINGS, OR APPROVED BY ENGINEER		• FURNISH ALL END AND INTERMEDIATE STIFFENERS, BLOCKING AND/OR SHEAR PANELS, METAL BRIDGING ASSEMBLIES AND HANGERS,	Z O
OF RECORD. 5. PLACE ELECTRICAL CONDUIT NEAR CENTER OF ELEVATED SLAB.		AS REQUIRED TO PROVIDE A COMPLETE FLOOR OR ROOF STRUCTURAL SYSTEM. TOP AND BOTTOM CHORDS OF TJI JOISTS SHALL BE MANUFACTURED FROM LVL MATERIAL AND SHALL BE EQUAL TO OR GREATER DIMENSION THAN THE TRUS JOIST, A WEYERHAEUSER	
6. SPLAY REINFORCING AROUND SLAB OPENINGS WITH 1" IN 10" SPLAY, UNLESS NOTED OTHERWISE.		BUSINESS SERIES INDICTED ON THE DRAWINGS. DEPTH(S) OF JOIST(S) OR JOIST(S) SPACING MAY NOT BE CHANGED WITHOUT PRIOR WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD.	⊢ ⊢
<ul> <li>7. MINIMUM COVER FROM CONCRETE SURFACES TO REINFORCING:</li> <li>3 ± 1/2" TO BOTTOM OF FOOTING</li> </ul>		<ul> <li>4. PARALLEL STRAND LUMBER "PSL".</li> <li>BEAM, HEADER, BLOCKING: 2.0E GRADE OTHERWISE NOTED ON PLANS</li> </ul>	► ►
<ul> <li>2" ± 1/4" TO EARTH FACE OF WALL</li> <li>1" ± 1/4" TO INSIDE FACE OF WALL</li> </ul>		<ul> <li>5. LAMINATED STRAND LUMBER "LSL".</li> <li>RIM BOARD: 1<sup>1</sup>/<sub>4</sub>" WIDE, 1.3E GRADE OTHERWISE NOTED ON PLANS.</li> </ul>	RA VA
3/4" SLAB TO TOP AND BOTTOM SURFACES		<ul> <li>RIM BOARD: 1% WIDE, 1.3E GRADE OTHERWISE NOTED ON PLANS.</li> <li>BEAM, HEADER, BLOCKING: 1.5E GRADE OTHERWISE NOTED ON PLANS.</li> </ul>	
<ol> <li>CENTER OF SLABS-ON-GRADE</li> <li>REINFORCING LAP SPLICES: CONFORM WITH ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", 2-FEET, UNLESS</li> </ol>		<ul> <li>STUDS: 1½" WIDE, 1.5E GRADE.</li> <li>FOUNDATION SILL PLATES: STRANDGUARD 1.3E GRADE</li> </ul>	
NOTED OTHERWISE ON DRAWINGS: CAST-IN-PLACE CONCRETE		<ol> <li>SLOPED BEARING REQUIREMENTS: JOIST SUPPLIER AND CONTRACTOR TO COORDINATE.</li> <li>DOUBLE ALL JOISTS UNDER MECHANICAL UNITS, UNLESS NOTED OTHERWISE.</li> </ol>	
1. PROVIDE CONCRETE MATERIALS, FORM WORK, MIXING, PLACING AND CURING ACCORDING TO ACI 301, "STANDARD SPECIFICATION FOR		<ol> <li>DO NOT NOTCH OR DRILL STRUCTURAL MEMBERS, EXCEPT AS APPROVED BY THE ENGINEER OF RECORD PRIOR TO INSTALLATION.</li> </ol>	
STRUCTURAL CONCRETE". 2. CONCRETE MIX DESIGNS: F' <sub>C</sub> = 3,000 PSI		FASTENING AND CONNECTIONS 1. PROVIDE THE MINIMUM NUMBER OF FASTENERS PER THE FASTENER SCHEDULE FOR WOOD MEMBERS, UNLESS NOTED OTHERWISE ON	Q Z ⊢
2. CONCRETE MIX DESIGNS: F' <sub>C</sub> = 3,000 PSI ANCHORS IN CONCRETE		<ol> <li>2. SIMPSON STRONG-TIE CONNECTORS IN CONTACT WITH PRESERVATIVE-TREATED WOOD, FIRE-RETARDANT-TREATED WOOD OR EXPOSED</li> </ol>	
CAST-IN-PLACE ANCHORS SHALL BE ACCURATELY AND SECURELY PLACED.     ANCHOR BOLTS; BOLTS WITH ROLLED THREADS, ANCHOR BOLT NUTS: CONFORM WITH ASTM A194, ASTM A307 MATERIAL HOT-DIPPED		2. SIMPSON STRONG-THE CONNECTORS IN CONTACT WITH PRESERVATIVE-TREATED WOOD, FIRE-RETARDANT-TREATED WOOD OR EXPOSED TO EXTERIOR SHALL BE ZMAX, HDG OR SST300. FOLLOW SIMPSON STRONG-TIE RECOMMENDATIONS FOR SELECTING CORROSION RESISTANT CONNECTORS.	<u>  8 s щ</u>
<ul> <li>GALVANIZED ACCORDING TO ASTM A153.</li> <li>UNLESS NOTED OTHERWISE ON PLANS PROVIDE <sup>5</sup>/<sub>8</sub>" Ø x 7" EMBEDMENT WITH <sup>1</sup>/<sub>4</sub>" x3" SQ PL WASHERS AT MAXIMUM 72" ON-CENTER</li> </ul>		3. PROVIDE GRACE VYCOR® DECK PROTECTOR IN-LIEU-OF CORROSION RESISTANT TREATMENTS FOR CONNECTORS IN CONTACT WITH	ΟΥ Π ΟΥ
SPACING AT FOUNDATION SILL PLATES. SIMPSON STRONG-TIE MASA OR MASAP MUDSILL ANCHORS MY BE USED IN-LIEU-OF ANCHOR BOLTS AND PLATE WASHERS. SEE SHEARWALL SCHEDULE FOR SPACING.		PRESERVATIVE-TREATED WOOD, FIRE-RETARDANT-TREATED WOOD AT INTERIOR APPLICATIONS, UNLESS NOTED OTHERWISE ON PLANS. 4. FASTENERS IN PRESERVATIVE-TREATED OR FIRE-RETARDANT-TREATED WOOD SHALL COMPLY WITH ASTM A153 AND SHALL BE	μ
2. INSTALL ACCORDING TO MANUFACTURER'S RECOMMENDATIONS & WITH IBC SPECIAL INSPECTION ACCORDING TO SPECIAL INSPECTION		HOT-DIPPED ZINC-COATED GALVANIZED STEEL, STAINLESS STEEL, SILICON BRONZE OR COPPER. 5. NAIL TYPE: COMMON OR SINKER, UNLESS NOTED OTHERWISE ON DRAWINGS.	
<ul> <li>MECHANICAL ANCHORS: ICC-APPROVED; CONFORM WITH FF-S-325, GROUP II, TYPE 4, CLASS 1. MATERIAL: (ZINC PLATED ACCORDING TO ASTM B 633, HOT-DIPPED GALVANIZED ACCORDING TO ASTM A 153, OR AISI 304 STAINLESS STEEL). UNLESS NOTED OTHERWISE</li> </ul>		6. SEE FASTENER SCHEDULE FOR FASTENER SIZE AND LOCATION.	
ON PLANS EMBED MECHANICAL ANCHORS 4" MINIMUM INTO CONCRETE.		PLYWOOD SHEATHING CONNECTIONS 1. ROOF SHEATHING	
ACCEPTABLE ANCHORS: "KWIK-BOLT TZ", BY HILTI FASTENING SYSTEMS, INC, "STRONG BOLT 2" BY SIMPSON STRONG-TIE COMPANY, INC.,		BLOCK ALL EDGES WITH 2x4 FLATS	$ \Box \neq \infty \circ$
ADHESIVE ANCHORS (CONCRETE): ICC APPROVED. ANCHOR COMPONENTS: ALL-THREAD ROD, NUT, WASHER AND ADHESIVE     INJECTION GEL SYSTEM. ANCHOR RODS: RODS WITH ROLLED THREADS. ANCHOR ROD NUTS: CONFORM WITH ASTM A 194. ASTM A		<ul> <li>FASTENING: ALL NAILS COMMON UNLESS NOTED OTHERWISE</li> <li>AT EDGES OF EACH SHEET, BLOCKING &amp; WALLS8D AT 6" OC</li> </ul>	Г С П
36 MATERIAL HOT-DIPPED GALVANIZED ACCORDING TO ASTM A 153. • ACCEPTABLE ADHESIVE INJECTION GEL SYSTEMS:		<ul> <li>AT INTERIOR OF SHEETS</li></ul>	⊔ щ щ щ щ щ щ щ щ щ щ щ щ щ щ щ щ щ щ щ
•• "SET", BY SIMPSON STRONG-TIE COMPANY, INC.		2. FLOOR SHEATHING	
•• "HIT HY 200 SAFE", BY HILTI FASTENING SYSTEMS, INC. FRAMING LUMBER		<ul> <li>IMMEDIATELY PRIOR TO PLACING PANELS, APPLY A ¼" DIAMETER CONTINUOUS BEAD OF CONSTRUCTION ADHESIVE, CONFORMING WITH AFG-01, TO TOPS OF ALL JOISTS, BLOCKING AND PLATES.</li> </ul>	
1. LUMBER SPECIES: DOUGLAS FIR-LARCH GRADE LUMBER ACCORDING TO RULES OF WEST COAST LUMBER INSPECTION BUREAU (WCLIB). 2. LUMBER GRADES:		<ul> <li>FASTENING: ALL NAILS COMMON UNLESS NOTED OTHERWISE. SIMPSON STRONG-TIE "QUICK DRIVE" WSNTL212S SCREWS MAY BE USED IN-LIEU-OF COMMON NAILS, AT SAME SPACING INDICATED BELOW.</li> </ul>	
EXTERIOR WALL STUDS NO. 1		<ul> <li>AT EDGES OF EACH SHEET, BLOCKING &amp; WALLS10D AT 6" OC</li> <li>AT INTERIOR OF SHEETS</li></ul>	
INTERIOR BEARING WALL STUDS NO. 1     JOISTS NO. 1		•• AT BOUNDARIES OF FLOOR10D AT 6" OC 3. WALL SHEATHING	
BEAMS NO. 1     POSTS NO. 1		BLOCK ALL EDGES NOT SUPPORTED BY FRAMING MEMBERS WITH 2x4 FLATS, MIN.	
BLOCKING, PLATES, BRIDGINGSTANDARD OR BETTER OR STUD GRADE		<ul> <li>FASTENING: ALL NAILS COMMON UNLESS NOTED OTHERWISE</li> <li>AT EDGES OF EACH SHEET, BLOCKING &amp; WALLS</li></ul>	
<ol> <li>MAXIMUM MOISTURE CONTENT: 19% AT 3x OR LESS (LEAST DIMENSIONS) MEMBERS.</li> <li>PROVIDE SOLID BLOCKING (SAME DEPTH OF MEMBER) AT ALL POINTS OF BEARING (MAXIMUM SPACING OF 8'-0" ON-CENTER), AT JOISTS</li> </ol>		<ul> <li>AT INTERIOR OF SHEETS</li></ul>	
WITH A 5:1 OR GREATER DEPTH-TO-THICKNESS RATIO OR WHERE ONE EDGE OF JOIST IS NOT ATTACHED TO SHEATHING, WALLBOARD, BRACING, ETC.			
<ol> <li>MEMBER DIMENSIONS INDICATED ARE STANDARD NOMINAL UNLESS NOTED OTHERWISE.</li> <li>WOOD IN CONTACT WITH CONCRETE OR MASONRY ACCORDING TO AWPA STANDARD C-2. LABEL PRESERVATIVE-TREATED LUMBER WITH</li> </ol>		LIGHT-METAL PLATE-CONNECTED WOOD TRUSSES 1. DESIGN TRUSS SYSTEM ACCORDING TO PROVISIONS OF "DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE CONNECTED WOOD	
THE AWPB (AMERICAN WOOD PRESERVERS BUREAU) QUALITY MARK. 7. DOUBLE ALL FLOOR JOISTS UNDER ALL PARALLEL PARTITIONS.		TRUSSES", BY THE TRUSS PLATE INSTITUTE, UNLESS NOTED OTHERWISE. 2. TRUSS DESIGN CRITERIA:	
<ol> <li>BOUBLE ALL FLOOR JOISTS UNDER ALL FARALLEE FARTHONS.</li> <li>SEE SCHEDULE AND DRAWINGS FOR FASTENING.</li> </ol>		ROOF TRUSSES:	
GLUE LAMINATED MEMBERS		<ul> <li>DEAD LOAD: 15 PSF</li> <li>SNOW LIVE LOAD: 25 PSF</li> </ul>	DL. STAR
1. MEMBER SPECIES: WESTERN; MEMBER GRADE: SIMPLE SPANS; 24F-V4; CONTINUOUS OR CANTILEVERED SPANS: 24F-V8.		<ul> <li>WIND UPLIFT LOAD: 18.1 PSF ZONE 1, 22.9 PSF ZONE 2, &amp; 44.7 PSF ZONE 3</li> <li>LIVE LOAD DEFLECTION: L/360. MAX</li> </ul>	The of WAS
<ol> <li>MATERIAL STANDARDS: ALLOWABLE STRESSES: AITC 117. ARCHITECTURAL APPEARANCE GRADE: AITC 110-2001. MANUFACTURE AND FABRICATION: AITC A190.1. FABRICATE WITH WATERPROOF GLUES. SHAPE TOP OF MEMBERS TO ROOF SLOPE. ADD LAMINATIONS AS</li> </ol>		FLOOR TRUSSES:	
REQUIRED FOR SHAPING.  PROVIDE STANDARD 3500 FOOT RADIUS CAMBER, UNLESS NOTED OTHERWISE ON DRAWINGS.  IDENTIFY MEMBERS WITH THE APA-EWS MARK OF AMERICAN WOOD SYSTEMS OR MEMBER INSPECTION IS REQUIRED BY AN INDEPENDENT TESTING		<ul> <li>DEAD LOAD: 15 PSF</li> <li>LIVE LOAD: 40 PSF AT FLOORS &amp; 60 PSF AT DECKS</li> </ul>	
LAB. ERECT MEMBERS ACCORDING TO AITC SPECIFICATIONS.		<ul> <li>LIVE LOAD DEFLECTION: L/600, MAX</li> <li>PROVIDE TRUSS AND COMPRESSIVE MEMBER LATERAL BRACING AND CONNECTIONS FOR CONSTRUCTION AND PERMANENT</li> </ul>	Pegistered with
PLYWOOD SHEATHING		LOADS, INCLUDING BRACING FOR WIND UPLIFT. 3. CONNECTION PLATE DESIGN: DEVELOP FULL DESIGN STRESS IN A MEMBER. PROVIDE A MINIMUM TRANSFER AT ANY MEMBER OF 2.000	SSIONAL ENGINE
<ol> <li>PLYWOOD MATERIAL: GRADE: C-D, UNLESS NOTED OTHERWISE. MANUFACTURED WITH EXTERIOR GLUE ACCORDING TO UNITED STATES PRODUCT STANDARD PS 1-83/ANSI AL 99.1. CONFORM WITH APA PRODUCT STANDARD PS 1-07. SHALL BEAR THE AMERICAN PLYWOOD</li> </ol>		POUNDS.	
ASSOCIATION (APA) TRADEMARK. SUBSTITUTION OF ORIENTED STRAND BOARD (OSB) FOR PLYWOOD IS ACCEPTABLE IF THE OSB: CONFORMS WITH APA PS 2-04, GRADE 2-M-W. MANUFACTURED WITH EXTERIOR GLUE. LOAD/SPAN RATING INDEX EQUAL TO PLYWOOD.		<ul> <li>4. SHOP DRAWINGS:</li> <li>INCLUDE ERECTION PLAN SHOWING LATERAL BRACING FOR TRUSS COMPRESSIVE MEMBERS AND REQUIRED BRACING</li> </ul>	REVISIONS
BEARS THE APA TRADEMARK. 2. PROVIDE PRESSURE-TREATED PLYWOOD WHERE INDICATED ON DRAWINGS. CONFORM WITH AWPA STANDARD C-9. MARK SHEETS WITH		CONNECTIONS.     SUBMIT TRUSS DESIGN CALCULATIONS SHOWING MEMBER FORCES AND COMBINED STRESSES.	1 03-DEC-19
AWPB  SHEATHING TYPES:		<ul> <li>SUBMIT ICC APPROVAL INFORMATION.</li> <li>SUBMIT CERTIFICATES FROM AN INDEPENDENT INSPECTION COMPANY ASSERTING THAT TRUSSES DELIVERED TO PROJECT SITE</li> </ul>	2 08-APR-20
•• ROOF SHEATHING: <sup>15</sup> / <sub>32</sub> " INDEX <sup>32</sup> / <sub>16</sub>		CONFORM WITH APPROVED SHOP DRAWINGS SUBMIT INSPECTION CERTIFICATES PRIOR TO START OF ERECTION. 6. CONNECT TRUSSES TO SUPPORTING MEMBERS WITH ONE SIMPSON H1 ANCHOR & TWO 10D TOE NAILS, UNLESS NOTED OTHERWISE.	
<ul> <li>FLOOR: <sup>3</sup>/<sub>4</sub>" INDEX <sup>48</sup>/<sub>24</sub> T&amp;G MINIMUM.</li> <li>WALLS: <sup>15</sup>/<sub>32</sub>" INDEX <sup>32</sup>/<sub>16</sub></li> </ul>		<ol> <li>CONNECT TRUSSES TO SUPPORTING MEMBERS WITH ONE SIMPSON HTANCHOR &amp; TWO TOD TOE NAILS, UNLESS NOTED OTHERWISE.</li> <li>DOUBLE TRUSSES UNDER MECHANICAL UNITS, UNLESS NOTED OTHERWISE.</li> </ol>	
<ol><li>PLYWOOD LAYOUT AND INSTALLATION: LAY OUT PLYWOOD SHEATHING WITH END JOINTS STAGGERED, UNLESS NOTED OTHERWISE. LAY OUT PLYWOOD TO ELIMINATE WIDTHS LESS THAN 2'-0" UNLESS ALL EDGES OF UNDERSIZED PIECES ARE SUPPORTED BY BLOCKING.</li></ol>			
PROVIDE PANEL SPACING ACCORDING TO APA RECOMMENDATIONS. BLOCK SHEAR WALL SHEATHING WITH 2x4 FLAT BLOCKING AT ALL EDGES. FASTEN ACCORDING TO SCHEDULE AND DRAWINGS.		DIGITAL SIGNATURE	DATE: 22 ILINI 10
4. PROTECT FLOOR AND ROOF SHEATHING FROM EXTREME WET CONDITIONS.			PROJECT NO: 10,0024
			PROJECT NO: 19-002A
			1 OF 12 SHEETS

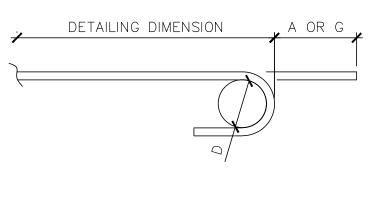
## SHEATH

## Permit Number: 20-04899

BAR	D A	180° H	OOKS	90° HOOKS
SIZE		A OR G	J	A OR G
#3	21⁄4"	5"	3"	6"
#4	3"	6"	4"	8"
<b>#</b> 5	3¾"	7"	5"	10"
#6	4½"	8"	6"	1'-0"
#7	5¼"	10"	7"	1'-2"
#8	6"	11"	8"	1'-4"
<b>#</b> 9	9½"	1'-3"	11 <i>3</i> /4"	1'-7"
#1O	10¾"	1'-5"	1'-1¼"	1'-10"
#11	12"	1'-7"	1'-2¾"	2'-0"
#14	18¼"	2'-3"	1'-9¾"	2'-7"
<i>#</i> 18	24"	3'-0"	2'-4½"	3'-5"

## DETAILING DIMENSION





			-
BAR SIZE	D <sup>A</sup>		MIC HOOK
		A OR G	НB
#3	1 1/2"	4 ¼"	3"
#4	2"	4 ½"	3"
#5	2 ½"	5 ½"	3 3/4"
#6	4 ½"	8"	4 ½"
#7	5 ¼"	9"	5 1/4"
#8	6"	10 ½"	6"
A) D = FI B) H DIME	NSION IS A	DE BEND (	E

HOOK DEVELOPMENT LENGTH f'c = 3,000 psi BAR SIZE #3 #4 #5 14 #6 117 #7 19

		.72	
	#4	2"	4½"
·	<b>#</b> 5	2½"	5½"
·	#6	4½"	8"
·	#7	5¼"	9"
N	#8	6"	10½"
DETAILING DIMENSION A OR G		NISHED INS NSION IS A	

BAR SIZE

#3

D <sup>A</sup>

1½"

NOTES:

#8

#9

#10

1. ALL LENGTHS ARE IN INCHES.

22

25

28

2. VALUES ARE BASED ON GRADE 60 REINFORCING BARS & NORMAL-WEIGHT CONCRETE.

BASIC PERMIT PACKAGE REVIEWED FOR CODE COMPLIANCE WITH IRC 2015 KITSAP COUNTY BUILDING DEPARTMENT

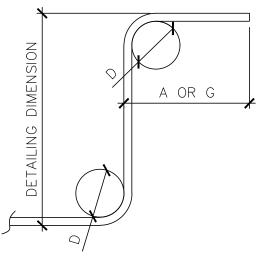
Reviewed for code compliance with IRC 2015 Kitsap County Building Department lasmith@co.kitsap.wa.us 11/16/2020

revised



## SEISMIC STIRRUP/TIE

STIRRUP AND TIE HOOKS				
A	13	35°	90°	
	A OR G	Н <sup>В</sup>	A OR G	
"	4"	2½"	4"	
	4½"	3"	4½"	
"	5½"	3¾"	6"	
"	8"	4½"	1'-0"	
<b>,,</b>	9"	5¼"	1'-2"	
10½" 6" 1'-4"				
D INSIDE BEND DIAMETER				



CHANGES MUST Be Approved Prior To Performing Work

Subject To Field Inspection

	ABBREVIATIONS						
AB	ANCHOR BOLT	EQ	EQUAL, EQUIVALENT	JT	JOINT	R	RADIUS
ACI	AMERICAN CONCRETE INST.	EQUIP	EQUIPMENT	L	ANGLE SECTION	REF	REFERENCE
ADD'L	ADDITIONAL	ES	EACH SIDE	LG	LONG	REINF	REINFORCING
ALT	ALTERNATE	EW	EACH WAY	LONGIT	LONGITUDINAL	REQ'D	REQUIRED
APPROX	APPROXIMATE	(E)	EXISTING	LL	LIVE LOAD	RW	RETAINING WALL
ARCH	ARCHITECTURAL	EXP	EXPANSION	LLH	LONG LEG HORIZONTAL	REV	REVISION
AT	ATTIC TRUSS	EXT	EXTERIOR	LLV	LONG LEG VERTICAL	SC	SLIP CRITICAL
BLDG	BUILDING	-1	COMPRESSIVE STRENGTH OF	LT	LIGHT	SCHED	SCHEDULE
BLKG	BLOCKING	f'c	CONCRETE, PSI	МАХ	MAXIMUM	SHTG	SHEETING, SHEATHING
ВМ	BEAM	FDN	FOUNDATION	МВ	MACHINE BOLT	SIM	SIMILAR
BRG	BEARING	FIN	FINISH	МС	MISC CHANNEL SECTION	SECT	SECTION
вот	воттом	FF	FAR FACE, FINISH FLOOR	MANF	MANUFACTURED	SOG	SLAB ON GRADE
С	CHANNEL SECTION	FLR	FLOOR	MECH	MECHANICAL	SPECS	SPECIFICATIONS
CGS	CENTROID OF TENDON	FLG	FLANGE	MEZZ	MEZZANINE	SQ	SQUARE
CJ	CONTROL JOINT	FOC	FACE OF CONCRETE	MIN	МІЛІМИМ	SS	STAINLESS STEEL
Ę	CENTERLINE	FOM	FACE OF MASONRY	MISC	MISCELLANEOUS	STAG	STAGGER(ED)
CLR	CLEAR	FOS	FACE OF STEEL, STUD	NF	NEAR FACE	STD	STANDARD
СМИ	CONCRETE MASONRY UNIT	FS	FAR SIDE	NIC	NOT IN CONTRACT	STIFF	STIFFENER
COL	COLUMN	FTG	FOOTING	NO or #	NUMBER	STL	STEEL
CONC	CONCRETE	GA	GAUGE	NS	NEAR SIDE	STRUCT	STRUCTURAL
CONN	CONNECTION	GALV	GALVANIZED	NOM	NOMINAL	SW	SHEARWALL
CONST	CONSTRUCTION	GAT	GIRDER ATTIC TRUSS	NTS	NOT TO SCALE	SYMM	SYMMETRICAL
CONT	CONTINUOUS, CONTINUITY	GRD	GRADE	OC	ON CENTER	Т	MANUFACTURED WOOD TRUSS
CONT'D	CONTINUED	GT	GIRDER TRUSS	OD	OUTSIDE DIAMETER	(T)	TOP
CSK	COUNTERSINK	HD	HOLDOWN	OPNG	OPENING	Т&В	TOP & BOTTOM
DBL	DOUBLE	HDG	HOT DIPPED GALVANIZED	OPP	OPPOSITE	THRD	THREAD
DET	DETAIL	HDR	HEADER	PARA	PARALLEL	ТОС	TOP OF CONCRETE
Ø	DIAMETER	HGR	HANGER	PCF	POUNDS PER CUBIC FOOT	TOS	TOP OF STEEL
DIM	DIMENSION	HORIZ	HORIZONTAL	PEN	PANEL EDGE NAILING	TYP	TYPICAL
DL	DEAD LOAD	HS	HIGH STRENGTH	PERP	PERPENDICULAR	UNO	UNLESS NOTED OTHERWISE
DO	DITTO	HSS	HOLLOW STRUCTURE STEEL	PL	PLATE	VERT	VERTICAL
DT	DRAG TRUSS	НТ	HEIGHT OR HIP TRUSS	PROJ	PROJECTION	W	WIDE FLANGE SECTION
DWL	DOWEL	IBC	INTERNATIONAL BUILDING CODE	PSF	POUNDS PER SQUARE FOOT	W/	WITH
EA	EACH	INFO	INFORMATION	PSI	POUNDS PER SQUARE INCH	W/O	WITHOUT
EF	EACH FACE	INFO	INFORMATION	PT	PRESERVATIVE TREATED	WWF	WELDED WIRE FABRIC
ELEV	ELEVATION	INT	INTERIOR				

FASTENER SCHEDULE			
CONNECTION	FASTENING	LOCATION	
JOIST TO SILL OR GIRDER	(3) 8D	TOE NAIL	
BRIDGING TO JOIST	(2) 8D	TOE NAIL EACH END	
SOLE PLATE TO JOIST OR BLKG	16D AT 16" OC	TYPICAL FACE NAIL	
SOLE PLATE TO JOIST OR BLKG	(3) 16D AT 16" OC	SHEARWALL - FACE NAIL	
TOP PLATE TO STUD	(2) 16D	END NAIL	
STUD TO SOLE PLATE	(2) 16D	END NAIL	
STUD TO 3x SOLE PLATE	(2) 20D	END NAIL	
BUILT-UP CORNER STUDS	16D AT 12" OC		
MULTIPLE STUDS	16D AT 12" OC	FACE NAIL	
DBL TOP PLATE	16D AT 16" OC	FACE NAIL	
DBL TOP PLATE - LAP SPLICE	(8) 16D	FACE NAIL (EACH SIDE)	
BLKG JOISTS OR RAFTERS TO TOP PLATE	(3) 8D		
RIM JOIST TO TOP PLATE	8D AT 6" OC	TOE NAIL	
CONTINUOUS HEADER, TWO PIECES	16D AT 16" OC	ALONG EDGE	
CEILING JOISTS TO PLATE	(3) 8D	TOE NAIL	
CONTINUOUS HEADER TO STUD	(4) 8D	TOE NAIL	
CEILING JOISTS, LAPS OVER PARTITIONS	(3) 16D	FACE NAIL	
CEILING JOISTS .TO PARA RAFTERS	(3) 16D	FACE NAIL	
RAFTER TO PLATE	(3) 8D COMMON	TOE NAIL	
BUILT-UP GIRDER, FLR JOISTS, AND BEAMS	(2) 16D AT 12" OC CLINCHED	FACE NAIL AT TOP & BOT STAGGERED ON OPPOSITE SIDES EQUAL 6" OC	
COLLAR TIE TO RAFTER	(3) 10D	FACE NAIL	
JACK RAFTER TO HIP	(3) 10D	TOE NAIL	
JACK RAFTER TO HIP	(2) 16D	FACE NAIL	
ROOF RAFTER TO 2x RIDGE BM	(2) 16D	TOE NAIL	
JOIST TO RIM JOIST	(3) 16D	FACE NAIL	
LEDGER STRIP	(3) 16D	FACE NAIL	

Permit Number: 20-04899

#	P	DST S	CHEDULE
MARK	SIZE	SPECIES & GRADE	COMMENTS
1	6x6	DF-L NO. 1	
2	(2) 2x4	DF-L NO. 1	
3	(2) 2x6	DF-L NO. 1	
4	4×4	DF-L NO. 1	
5	4x6	DF-L NO. 1	
6	PT 4x4	HF NO. 1	

BEAF	RING WALL
	SCHEDULE
SIZE	SPECIES & GRADE

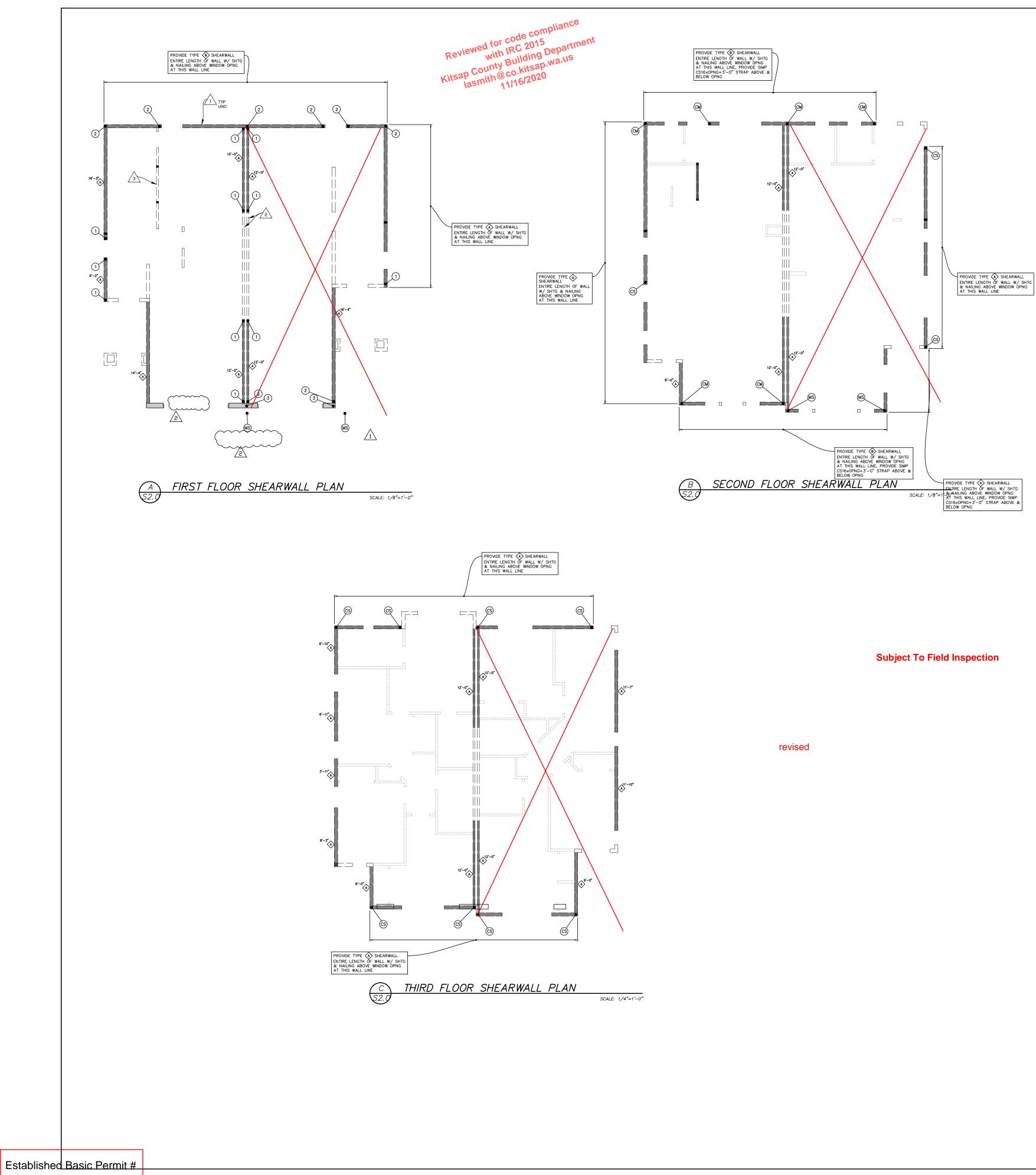
MARK	SIZE	SPECIES & GRADE
1	2x6 @ 16" OC	DF-L NO. 2
2	2x4 @ 16" OC	DF-L NO. 2
3	2x4 @ 8" OC	DF-L NO. 2

#

(F#)	FOC	TING	SCHE	EDULE
			1	

MARK	SIZE	REINFORCING
F1.6	1'-6" SQx0'-10"	#4 @ 8" OC EW
F2	2'-0" SQx0'-10"	#4 @ 8" OC EW
F3.2	3'-2" SQx0'-10"	#4 @ 8" OC EW

PROJECT	W AN	RED BARN LANE - DUPLEX 1880/1620A	PACIFIC NORTHWEST
22-J <sup>NO:</sup> 1 DF 12	P OF W ACCOUNTS OF THE STORE	NW HOGAN LN & NELS NELSON RD NW	STRUCTURAL GROUP, INC
9-0	303 STERED VAL EN	REMERTON WA 98311	A PROFESSIONAL ENGINEERING COMPANY
-19 02A <b>1</b>	-		6193 NE MALBON CT. KINGSTON, WA 98346
ГS			360.903.2803



19-03671R2

# HOLDOWN & TENSION STRAP SCHEDULE

MARK	HOLDOWN OR1	ANCHOR	POST <sup>3</sup>	FOC	DTING	CAPACITY
WARK	TENSION STRAP	ROD <sup>2</sup>	P031	SIZE	REINF'G	(LBS)
1	HDU2-SDS2.5	‰"øx1'−6" EMBED	(2) 2x	N/A	N/A	3,075
2	HDU5-SDS2.5	5%"∅x1'−3" EMBED	(2) 2x	2'-0" SQx1'-0"	#4×1-8 @ 12" OC	5,645
CS	CSx14-4'-0"	N/A	2x	N/A	N/A	2,490
СМ	CMST14x6'-0"	N/A	(2) 2x	N/A	N/A	6,490
MS	MSTC48B3	N/A	(2) 2x	N/A	N/A	3,975

## NOTES:

- 1. PROVIDE SIMPSON STRONG-TIE OR EQUIVALENT. FOR EQUIVALENT HOLDOWN OR STRAP, SUBMIT TO ENGINEER OF
- RECORD FOR APPROVAL. INSTALL PER MANUFACTURER SPECIFICATIONS. 2. PROVIDE ASTM A36 OR A307 THREADED ROD W/ PL ¼"x3" SQ W/ DBL NUTS @ END.
- 3. USE MINIMUM TWO STUDS AT END OF SHEARWALL. PROVIDE EDGÉ NAILING FOR FULL HEIGHT OF MULTIPLE STUDS OR POST AT TIE DOWN ANCHOR, DOOR AND WINDOW JAMBS.
- 4. PLACE HD NO CLOSER THAN 6" TO FND VENT OR OTHER CONC STEM WALL OPNG'S.

X		SHE	EARWA		SCHEDL	JLE	
MARK	WALL SHEATHING	FAS	TENING <sup>2</sup>	FOUNE	DATION SILL PLATE	SOLE PLATE FASTENERS	TOP PLATE FASTENERS
	SHEATTING	PANEL EDGE <sup>3</sup>	INTERMEDIATE SUPPORT	SIZE	FASTENER	FASTENERS	TASTENENS
A	<sup>15</sup> ⁄ <sub>32</sub> " SHTG	8d @ 6" OC	8d @ 12" OC	2x	5%"ø @ 60" OC	16d @ 8" OC	SIMP LS50 @ 24" OC
В	<sup>15</sup> ⁄ <sub>32</sub> " SHTG	8d @ 4" OC	8d @ 12" OC	Зx	5%"ø @ 40" OC	(2) 16d @ 12" OC	SIMP LS50 @ 16" OC
С	<sup>15</sup> ⁄ <sub>32</sub> " Shtg	8d @ 3" OC	8d @ 12" OC	Зx	5%"ø @ 32" OC	(3) 16d @ 12" OC	SIMP LS70 @ 16" OC

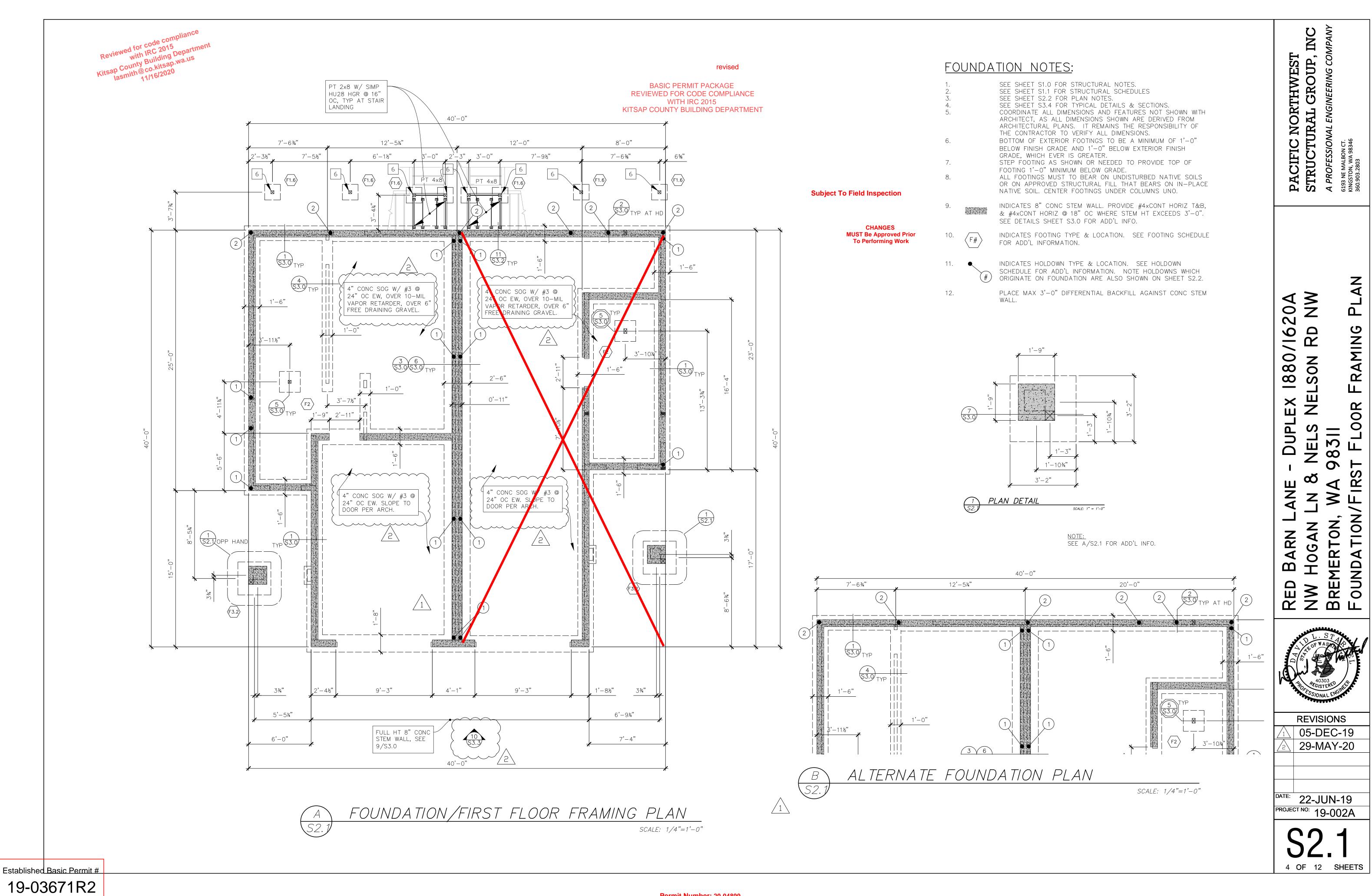
## NOTES:

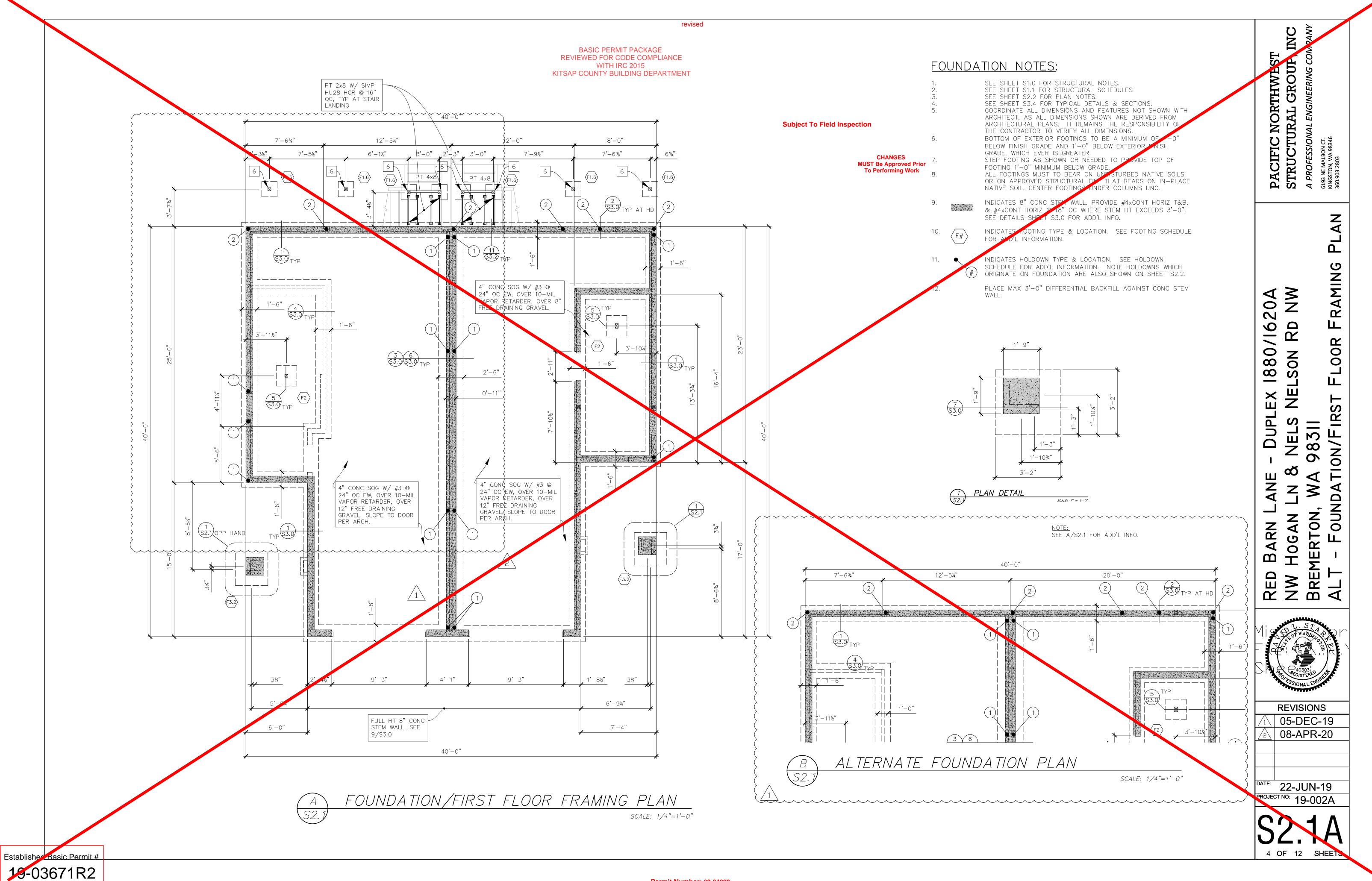
- 1. BLOCK ALL PANEL EDGES. SEE STRUCTURAL NOTES FOR SHTG REQUIREMENTS. SEE DETAIL 6/S3.3 FOR TYPICAL CONSTRUCTION.
- NAILS SHALL BE COMMON TYPE.
- PROVIDE EDGE NAILING AT ALL END STUDS, SILL PLATES, RIM BOARDS, AND TOP PLATES. STAGGER EDGE NAILING FOR FULL HEIGHT OF STUDS AT HOLD DOWN ANCHORS, DOOR JAMBS, AND WINDOW JAMBS. 4.
- 5. USE 3x STUDS OR DBL 2x STUDS FASTENED TOGETHER W/ (2) 10d NAILS @ 12" OC @ LOCATIONS RECEIVING EDGE NAILING FROM ABUTTING PANELS FOR SHEARWALL TYPE(S) B & C AT ALL FRMG LEVELS. 6. SILL PLATES AGAINST CONCRETE SHALL BE PRESERVATIVE-TREATED. SEE DETAIL 2/S3.3 FOR TYPICAL ANCHOR
- BOLT LAYOUT AND SILL PLATE NOTCHING REQUIREMENTS. 7. PROVIDE ASTM A307 ANCHOR BOLTS WITH 7" MIN EMBEDMENT FOR FOUNDATION SILL PLATES AGAINST CONC.
- APPROVED MECHANICAL ANCHORS MAY BE USED IN-LIEU-OF ANCHOR BOLTS. SEE STRUCTURAL NOTES FOR APPROVED MECHANICAL ANCHORS. PROVIDE PL¼"x3" SQ GALVANIZED WASHERS AT EACH ANCHOR BOLT OR MECHANICAL ANCHOR PLACE WALL SHEATHING ON SAME SIDE OF WALL AS WHERE SHEARWALL MARK IS LOCATED OR ARROW POINTS. 9. PROVIDE DOUG-FIR LARCH MEMBERS FOR ALL SHEARWALLS.
- 10. PROVIDE MINIMUM TWO (2) STUDS AT END OF SHEARWALL UNO. SEE HOLDOWN SCHEDULE FOR ADD'L POST SIZE INFORMATION.

CHANGES MUST Be Approved Prior To Performing Work

BASIC PERMIT PACKAGE REVIEWED FOR CODE COMPLIANCE WITH IRC 2015 KITSAP COUNTY BUILDING DEPARTMENT

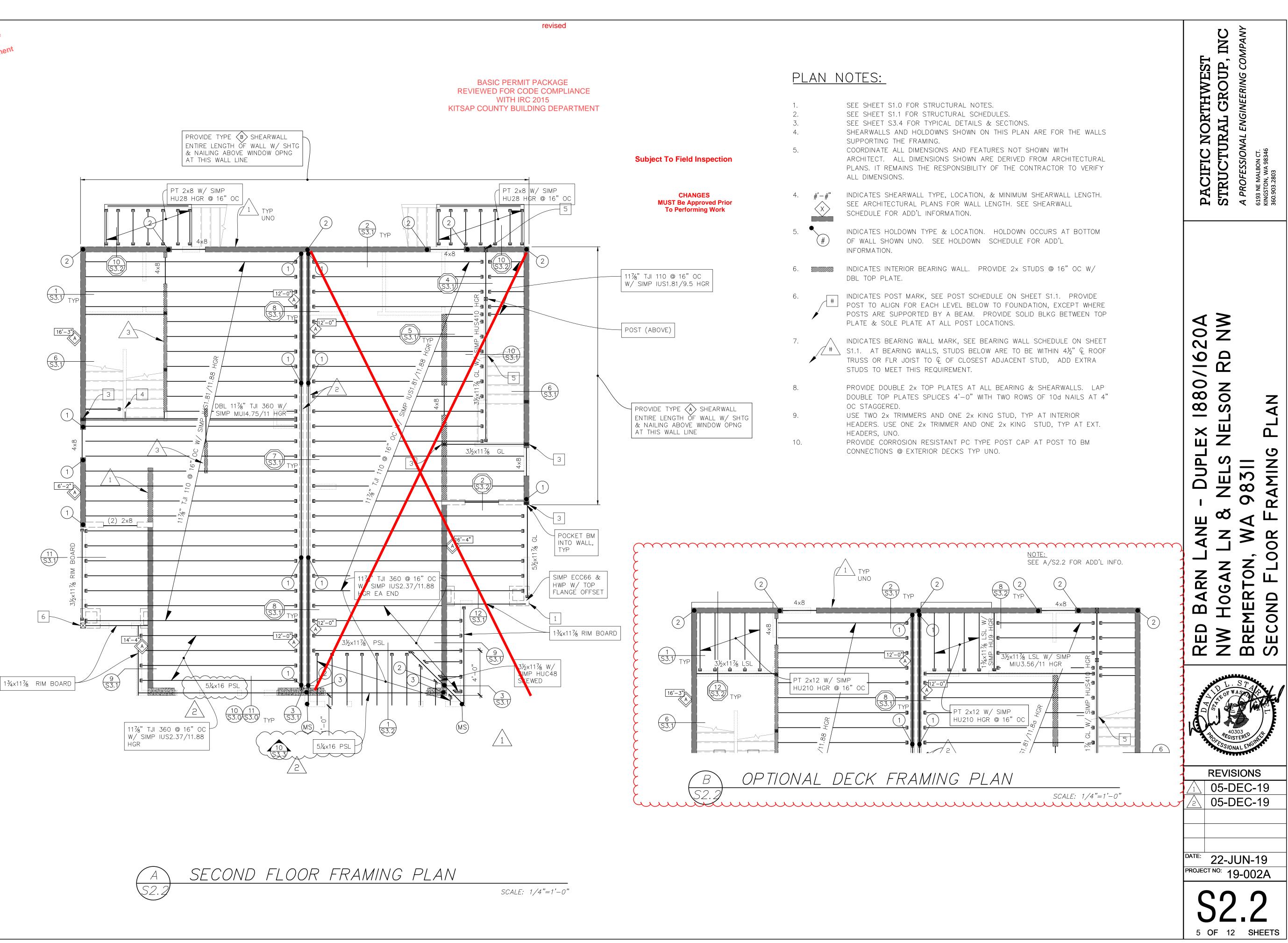
PROJECT			RED BARN LANE - DUPLEX 1880/1620A	PACIFIC NORTHWEST
		D OF ACTION	NW HOGAN LN & NELS NELSON RD NW	STRUCTURAL GROUP, INC
UN-1 9-002	SIONS	STERED 303 STERED VAL ENGIN	BREMERTON, WA 98311	A PROFESSIONAL ENGINEERING COMPANY
			SHEARWALL & HOLDOWN PLANS & SCHEDULES	810.903.2803 360.903.2803



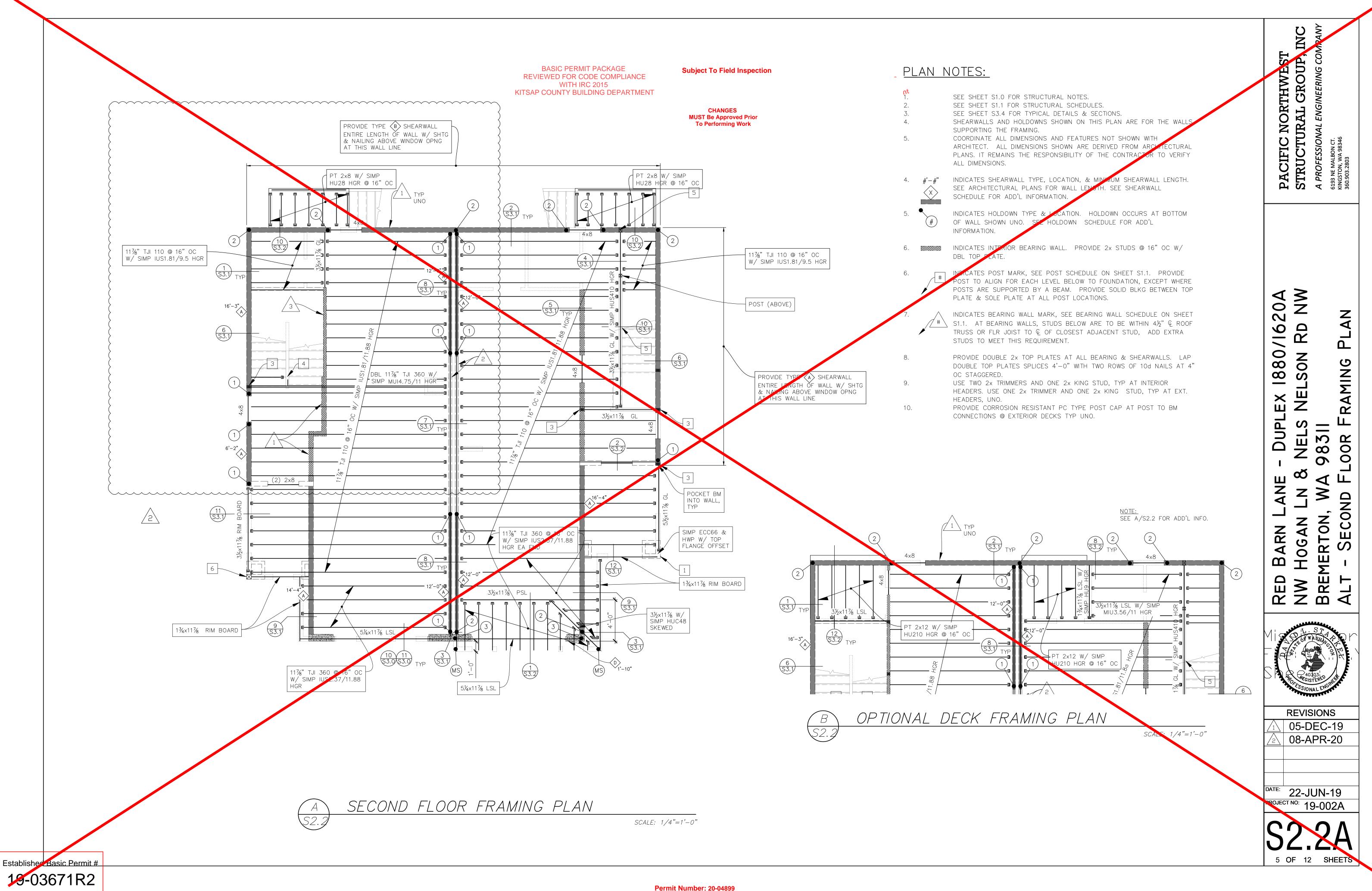


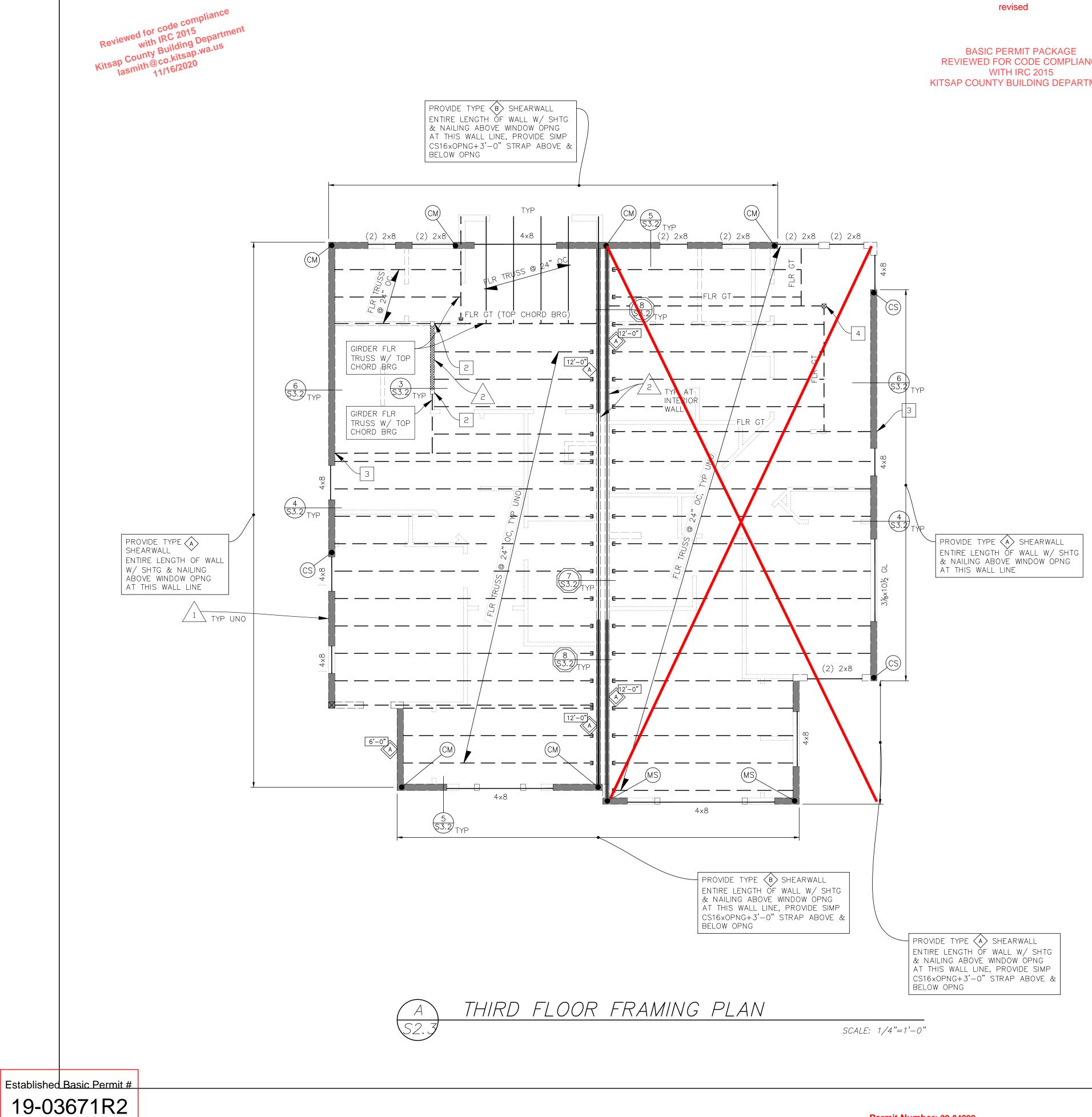


ed for code compliance with IRC 2015 کو County Building Departme Jasmith@co.kitsap.wa.us 11/16/2020



Established Basic Permit # 19-03671R2





revised

# BASIC PERMIT PACKAGE REVIEWED FOR CODE COMPLIANCE KITSAP COUNTY BUILDING DEPARTMENT

<u>PLAN NOTES:</u>

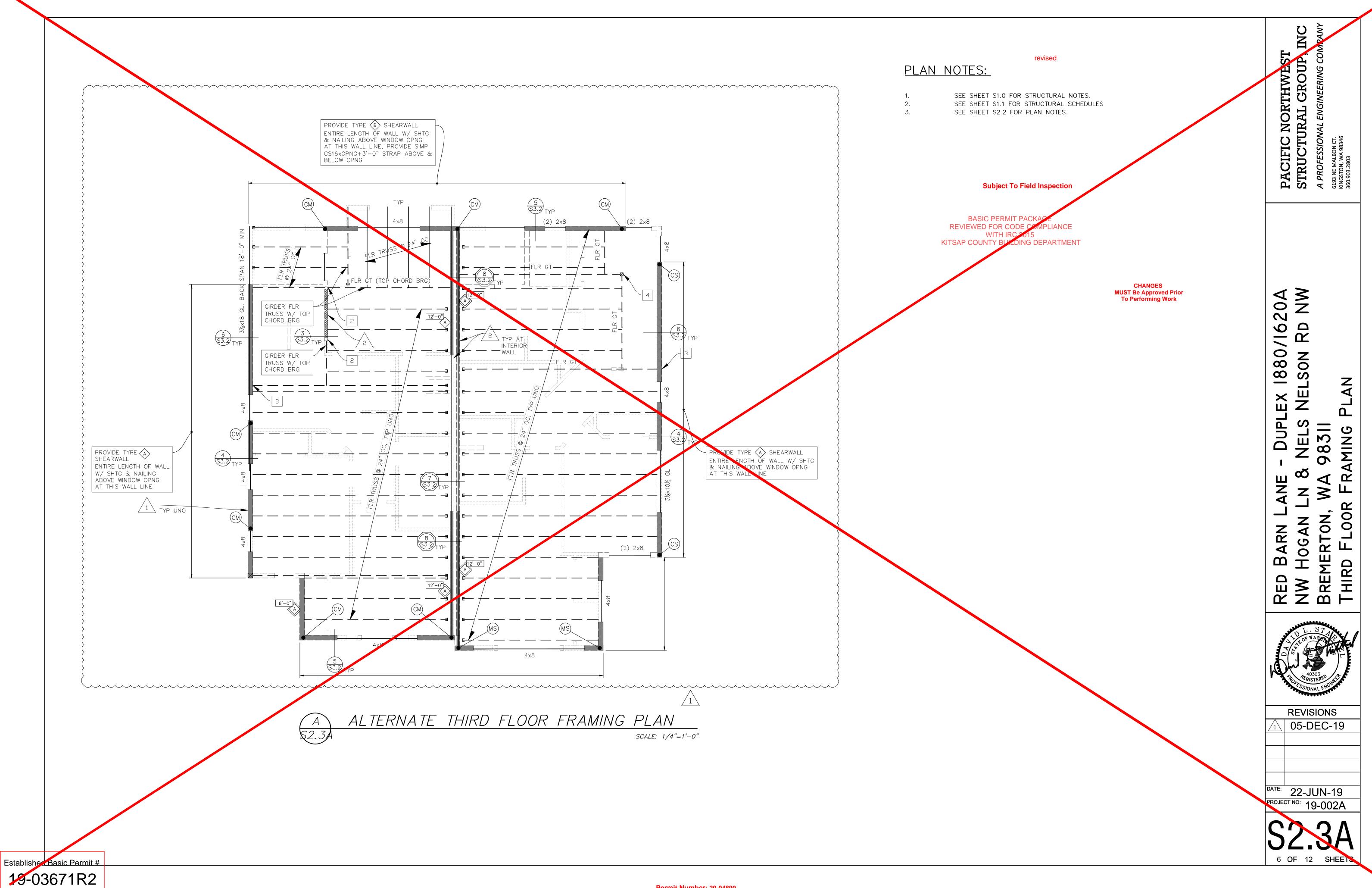
1.	SEE	SHEET	S1.0	FOR	ST
2.	SEE	SHEET	S1.1	FOR	STF
3.	SEE	SHEET	S2.2	FOR	ΡL

RED BARN LANE - DUPLEX 1880/1620A NW HOGAN LN & NELS NELSON RD NW BREMERTON, WA 98311 THIRD FLOOR FRAMING PLAN	RED BAR NW HOGA BREMERTO THIRD FL
Addition of the second	REVISIONS
	DATE: 22-JUN-19

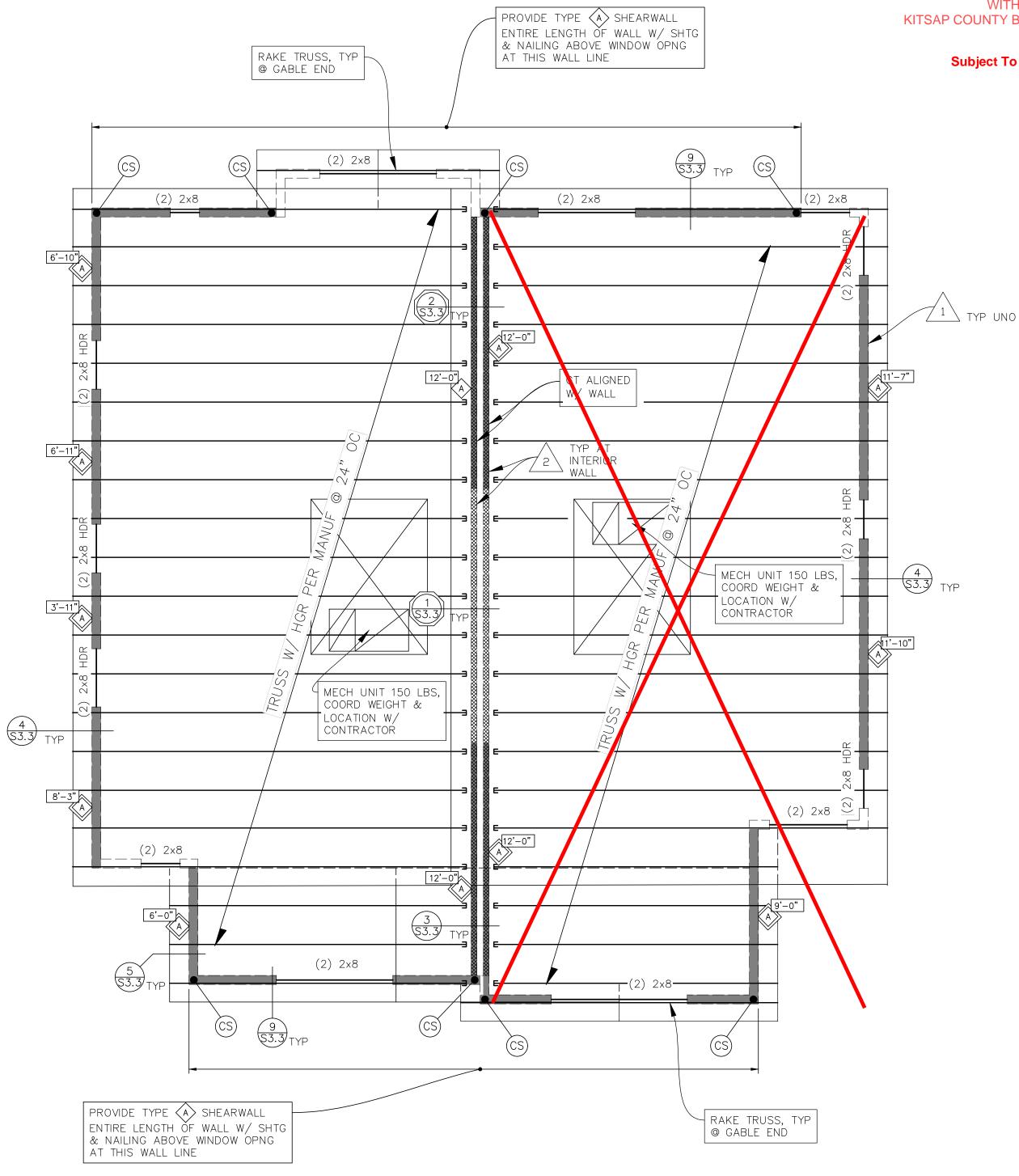
**Subject To Field Inspection** 

CHANGES MUST Be Approved Prior To Performing Work

TRUCTURAL NOTES. STRUCTURAL SCHEDULES PLAN NOTES.



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UPPER ROOF FRAMING PLAN A S2

Established Basic Permit # 19-03671R2 revised

## BASIC PERMIT PACKAGE REVIEWED FOR CODE COMPLIANCE WITH IRC 2015 KITSAP COUNTY BUILDING DEPARTMENT

## Subject To Field Inspection

## ROOF NOTES:

1. 2. 2. 3. 4.		SEE SHEET S1.0 FOR STRUCTURAL N SEE SHEET S1.1 FOR STRUCTURAL S SEE SHEET S3.4 FOR TYPICAL DETA SEE SHHET S2.2 FOR PLAN NOTES. SHEARWALLS AND HOLDOWNS SHOW SUPPORTING THE FRAMING.
5. 6.	" ⊤ "	COORDINATE ALL DIMENSIONS & FEA INDICATES LIGHT METAL PLATE TRUS PLATE WOOD TRUSSES SHALL BE SF ROOF SNOW LOAD OF 25 PSF & RC MECHANICAL WEIGHTS. DESIGN TRU ARHCITECTURAL SHEET(S), USE MUL MIN BOT CHORD LIVE LOAD (NEED N
7. 8. 9. 10.	"GT "	
11.		ALL TRUSSES SHALL BE DESIGNED CAPACITY OF STUD WALL TOP PLAT SUPPORTS IS ACCEPTABLE, IF CAPA
12. 13.		USE (1) 2x6 TRIMMER STUD & (1) 2 SHEARWALLS & HOLDOWNS ORIGINA

CHANGES MUST Be Approved Prior To Performing Work

SCALE: 1/4"=1'-0"

NOTES. SCHEDULES. AILS & SECTIONS.

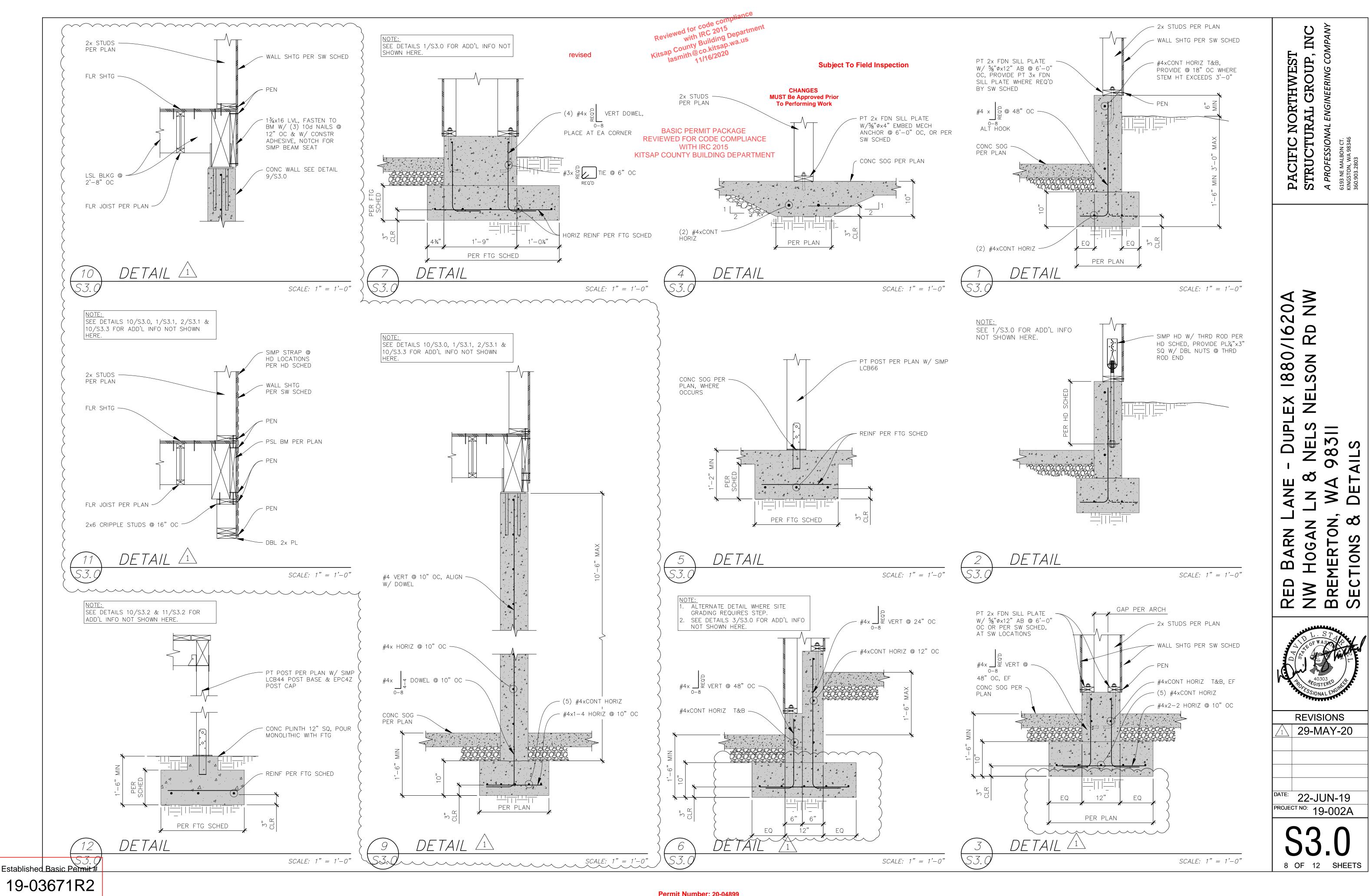
WN ON THIS PLAN ARE FOR THE WALLS

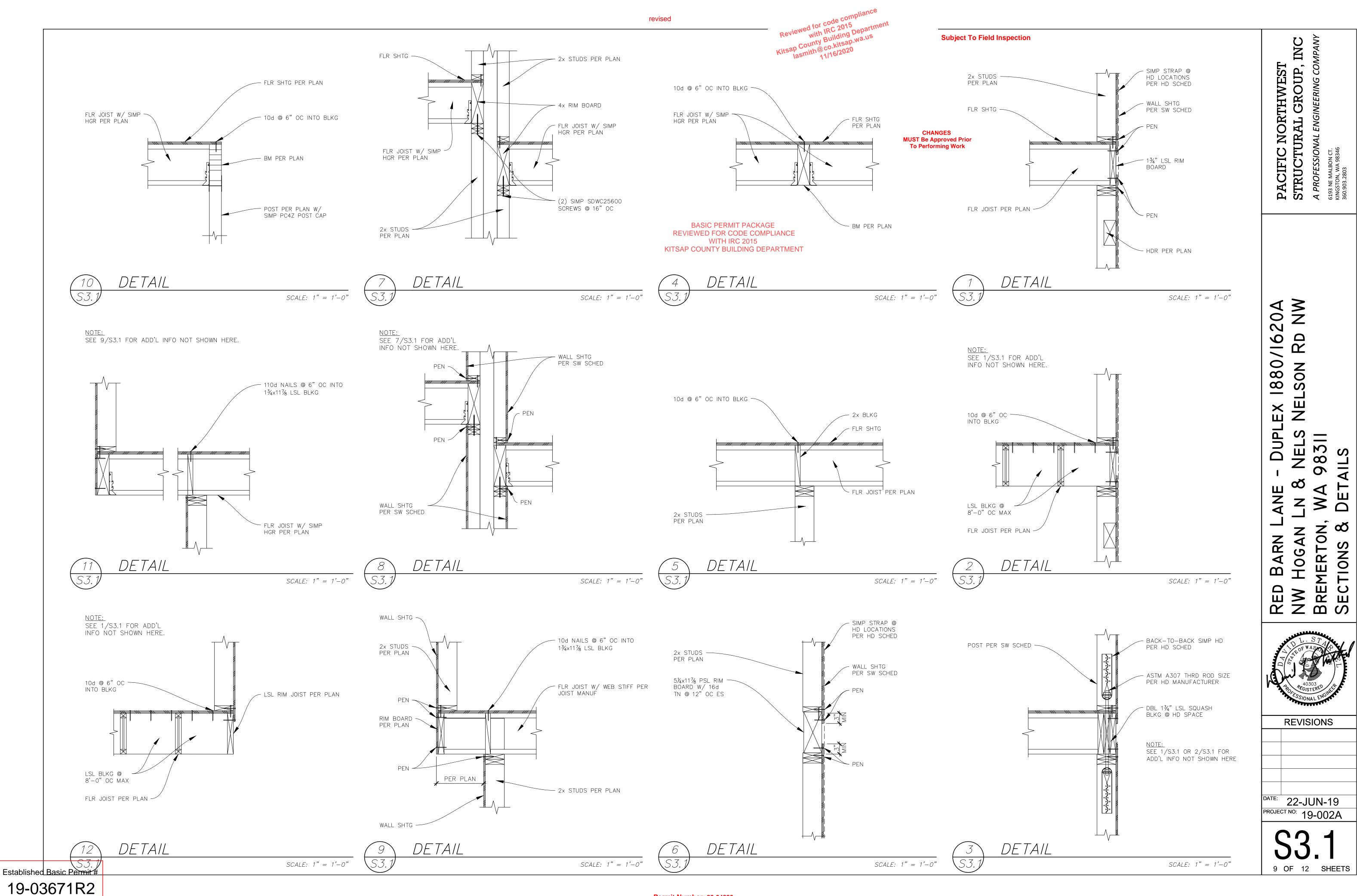
EATURES NOT SHOWN W/ ARCHITECT. JSS DESIGNED BY OTHERS. ALL LIGHT-METAL SPACED @ 24" OC UNO. DESIGN TRUSSES FOR COOF DEAD LOAD OF 15 PSF IN ADDITION TO USSES FOR ADD'L LOADS AS SHOWN ON ILTIPLE TRUSSES AS REQ'D. DESIGNED FOR 10 PSF NOT BE COMBINED W/ ROOF LIVE LOAD). MIN 2x6 BOT CHORD. W/ SIMPSON HUS26 UNO.

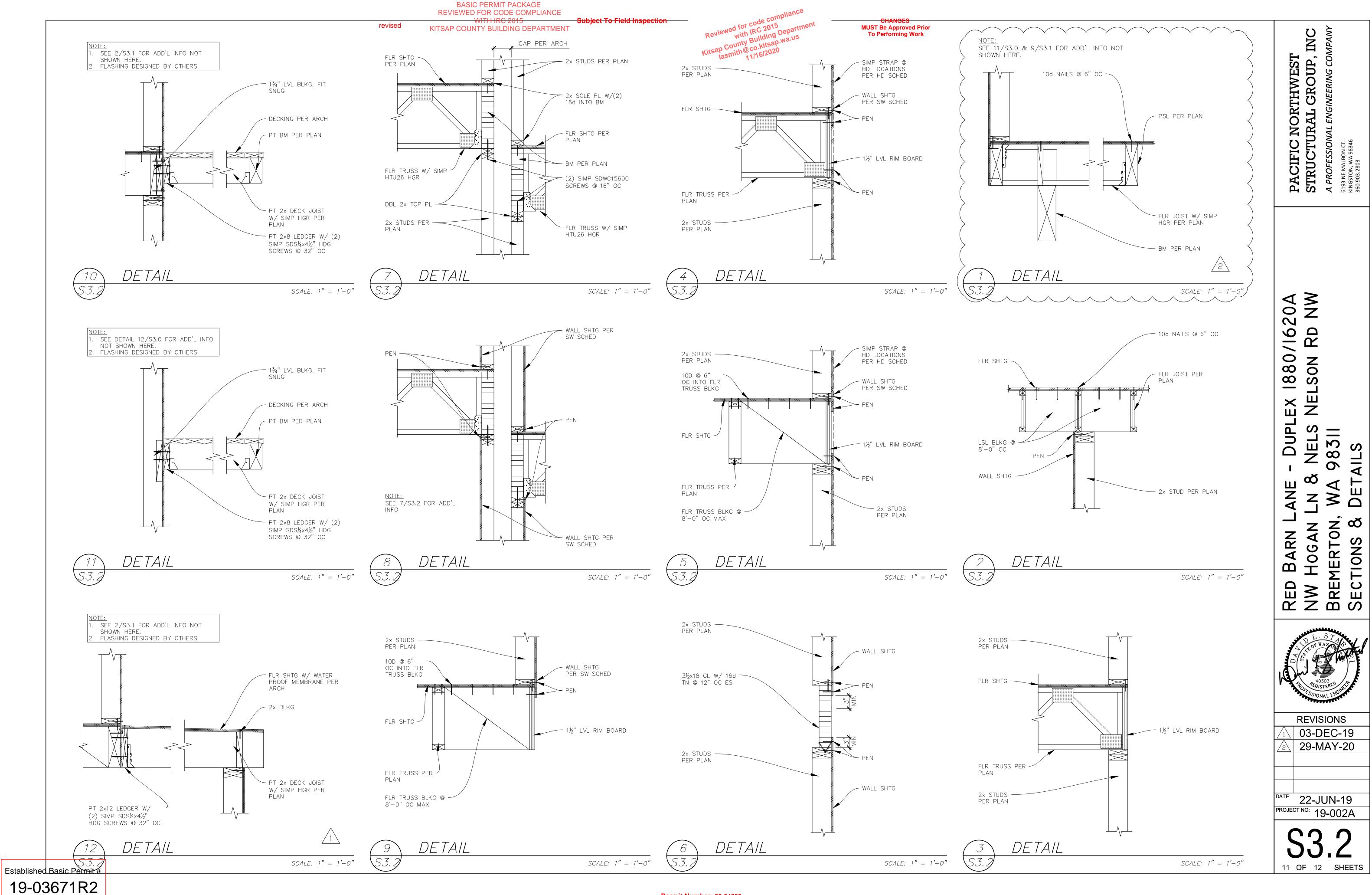
OTES. N SAME COUNT AS GT PLYS, BUT NOT LESS THAN FLOORS TO FOUNDATION. PROVIDE SOLID BLK'G

TO ACCOUNT FOR CROSS GRAIN BEARING TE, AS REQ'D. THE USE OF SIMPSON TBE ACITIES ARE ADEQUATE. 2x6 KING STUD AT EXTERIOR HEADERS UNO. ATE @ FLOOR BELOW.

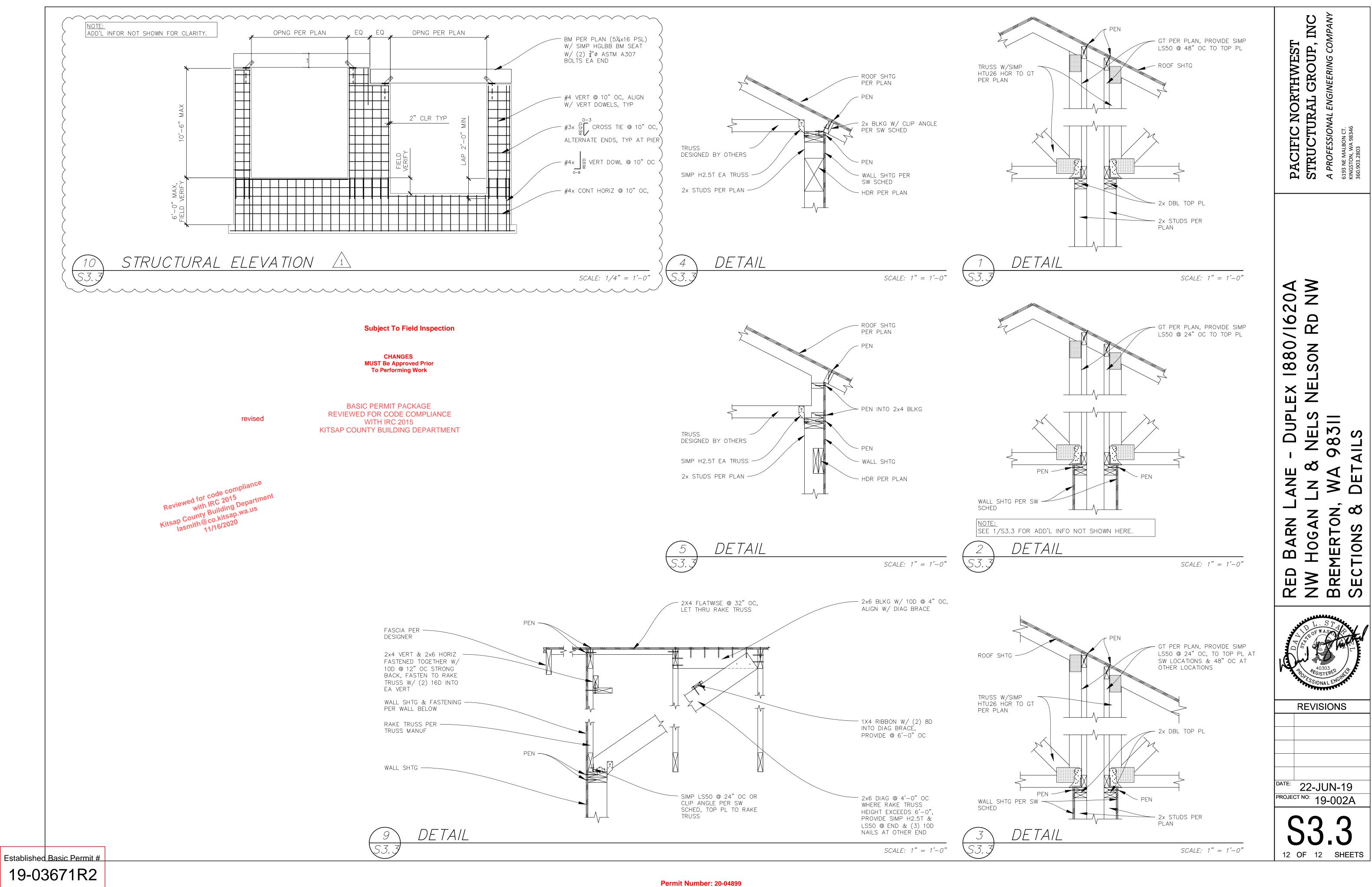


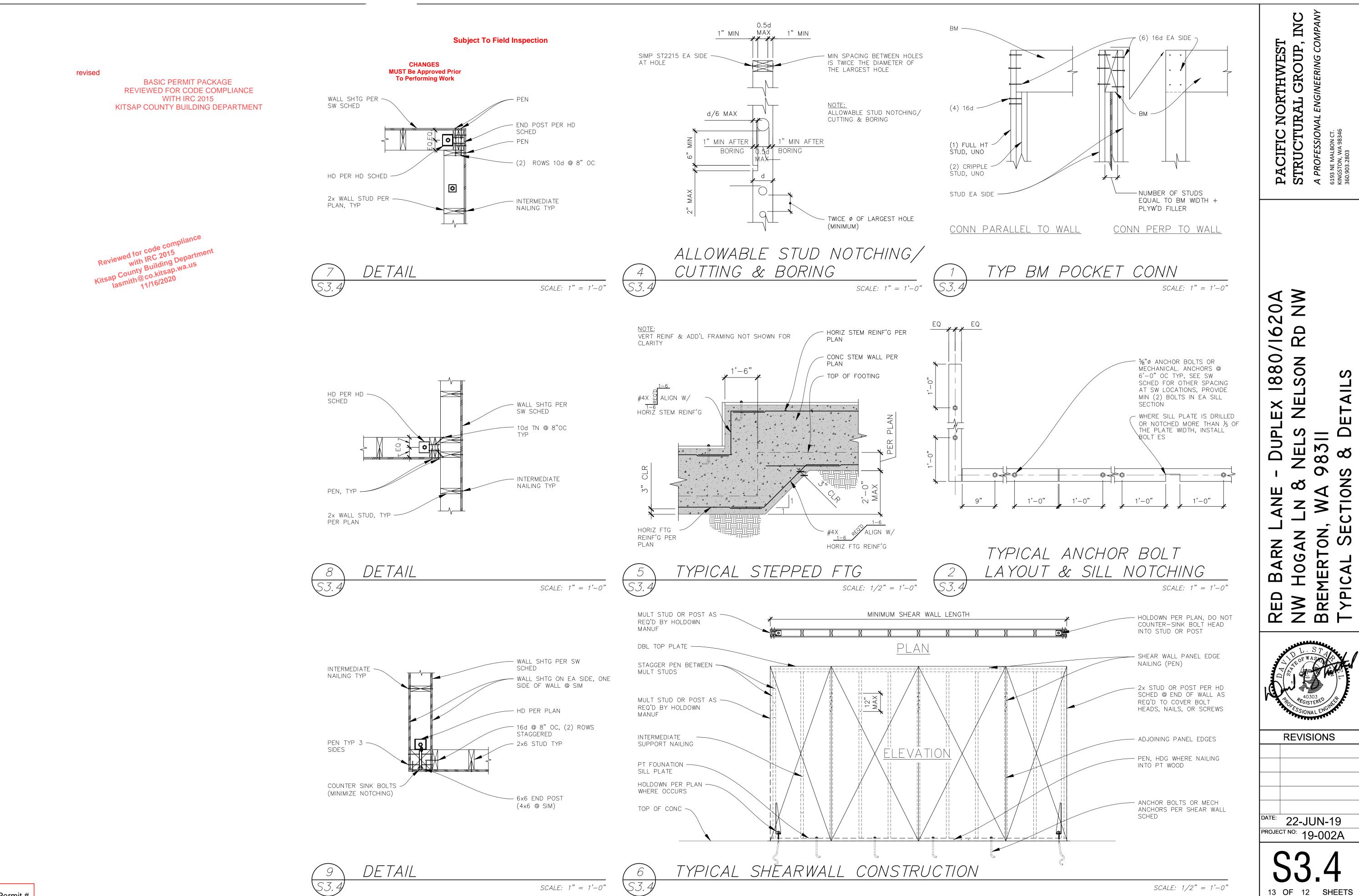






Permit Number: 20-04899





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Established Basic Permit # 19-03671R2

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